



**CASTLE  
GATE**  
COAL MINE

**MINING  
&  
RECLAMATION  
PLAN**

**PERMIT NO.  
ACT 007/004**

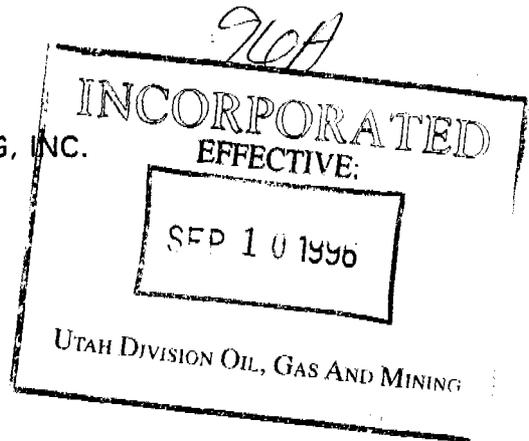
**VOLUME 6**

CHAPTER 7  
HYDROLOGY

CASTLE GATE MINE  
AMAX COAL COMPANY  
Carbon County, Utah

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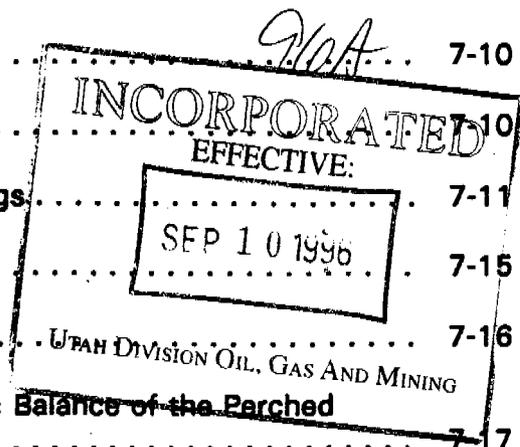
August 1996



CHAPTER 7  
 HYDROLOGY

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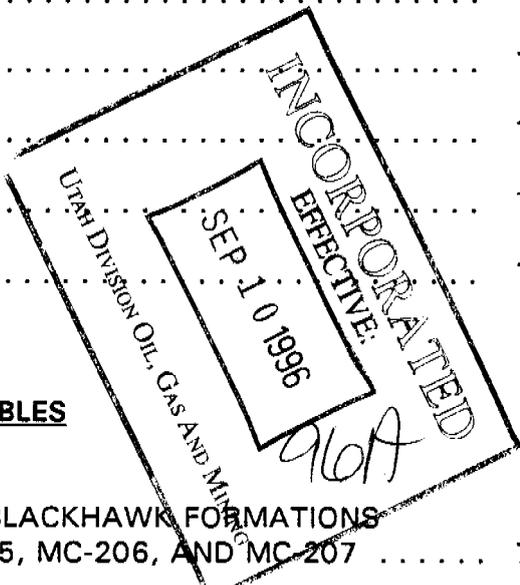
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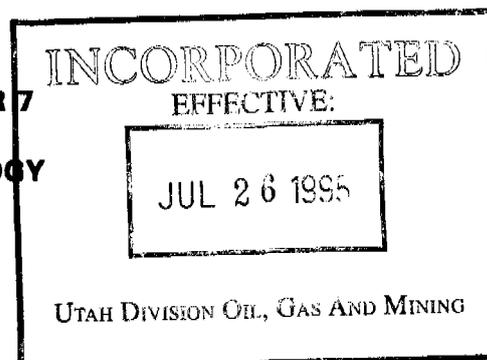
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CHAPTER 7  
HYDROLOGY



**7.1 GROUNDWATER HYDROLOGY**

This report has been prepared at the request of Price River Coal Company to respond to the State of Utah Division of Oil, Gas and Mining (DOGM) "Apparent Completeness Review (ACR) Comment UMC 783.15 Groundwater Information". In the above referenced comment, DOGM indicates,

Inadequacies in the description of the hydrogeologic system present at the Price River Mine Complex were a major topic of concern in the April 1981 ACR. To date these inadequacies have not been rectified... The applicant needs to provide a more detailed description of the hydrogeology of the area...

The purpose of this report is to provide the more detailed description of the hydrogeology of the Price River Coal Company mine plan and adjacent areas.

Castle Gate Coal Company took over operation of the mines in May of 1986. The mines, now owned by the Castle Gate Coal Company, will be referred to as the Castle Gate mines throughout this report. Reference will be made to appropriate exhibits submitted to DOGM as part of the Mining and Reclamation Plan.

**7.1-1 Regional Groundwater Hydrology**

The Castle Gate mine plan area is located along the western edge of the Book Cliffs coal field which extends from the Utah-Colorado state line to Castle Gate, Utah (see Figure 7-1). As indicated in Chapter 6 "Geology" of the Mining and Reclamation Plan, "the strata strike northwest to west and dip 3 to 6 degrees north into the Uinta basin". According to Spieker (1925), the strike of the strata coincides with the trend of the cliffs.

The geologic formations exposed on or adjacent to the mine plan area are either Upper Cretaceous or Tertiary (see Exhibit 7-1 and Exhibit 6-1).

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**Mancos Shale -** The Mancos Shale is a gray marine shale, locally heavily charged with carbonaceous material, slightly calcareous and gypsiferous (Doelling, 1972). The material (ranging from fine clay to siltstone) is nonresistant, forming flat desert surfaces and rounded hills and badlands.

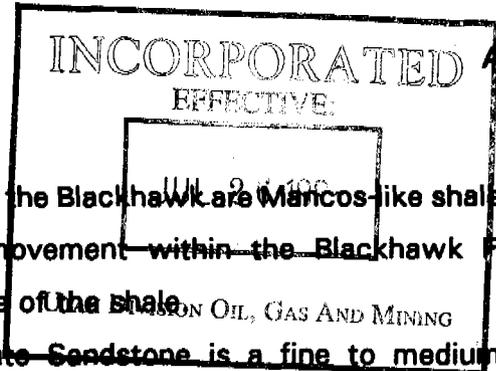
In the vicinity of the mine plan area, the Mancos Shale is separated into tongues by westward projecting littoral sandstone (Doelling, 1972). The sandstones are fine to medium grained, yellow gray to tan, medium bedded to massive, and cliff forming (Doelling, 1972).

Because of the marine environment associated with the formation of the Mancos Shale, groundwater percolating into the Mancos contacts soluble evaporites such as gypsum resulting in a rapid degradation of water quality. According to Freeze and Cherry (1979), if calcite and gypsum dissolve in fresh water, the water will become brackish with total dissolved solids of about 2100 to 2400 mg/l. If sufficient calcite and gypsum are present, the water will evolve quickly and directly to this degraded phase (Freeze and Cherry, 1979).

**Star Point Sandstone -** According to Doelling (1972), the Star Point Sandstone consists of massive, medium to light colored littoral sandstone tongues projecting easterly, separated by gray marine shale tongues projecting westerly. This formation, present only in the western portion of the Book Cliffs area, consists of three sandstone tongues (the Panther, Storrs and Spring Canyon, in ascending order). The Star Point Sandstone is relatively impermeable, with significant groundwater movement occurring on a regional level only as a result of secondary porosity, i.e. fractures.

**Blackhawk Formation -** Overlying the Star Point is the Blackhawk Formation which is the primary coal bearing formation of the Mesaverde Group. The Blackhawk consists of alternating sandstone, shale, and coal beds and ranges from 900 to 1300 feet thick in the Castle Gate areas, with the better coal beds assembled in the lower 500 feet (Doelling, 1972).

The sandstone is medium to fine grained, composed largely of semi-rounded quartz grains cemented by carbonate of lime and is reasonably well consolidated (Clark, 1928). Due to the cemented and consolidated nature of sandstones in the Blackhawk Formations, and apparent low specific yield (Cordova, 1964), water yielding capabilities of the Blackhawk are of only local importance.



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According to Doelling (1972), shales of the Blackhawk are Mancos-like shales. Shales act as effective barriers to groundwater movement within the Blackhawk Formation, particularly perpendicular to the bedding plane of the shale.

**Castlegate Sandstone** - The Castlegate Sandstone is a fine to medium grained, argillaceous to slightly calcareous normally massive bedded unit overlying the Blackhawk Formation (Doelling, 1972). Within the vicinity of the mine plan area, the unit is fluvial, becoming lagoonal toward the eastern portion of the Book Cliffs area. According to Clark (1928), the basal portion of the Castlegate consists of thin bedded rocks grading from sandy shale into sandstone, and in many locations shale and sandy shale are near the top of the member.

**Price River Formation** - The Price River Formation consists of yellow-gray to white, medium grained sandstone and shaley sandstone with gray to olive green shale (Doelling, 1972). The apparent low permeability of the Price River Formation (Cordova, 1964) reduces its water yielding capabilities except through fractures.

**North Horn Formation** - The North Horn Formation consists of a series of shale, sandstone, minor conglomerate and freshwater limestone (Doelling, 1972). Like the Blackhawk Formation, shales in the Castlegate, Price River, and North Horn Formations act as effective barriers to the vertical movement of water within these formations. Therefore, a significant portion of the water which reaches these formations probably percolates downward until encountering a shale layer, which then causes horizontal movement to the surface or another "drain" (i.e. sandstone finger within the formation).

**Flagstaff Limestone** - According to Doelling (1972), the Flagstaff Limestone consists of thin bedded limestones, shales and sandstones. The shales are interbedded with microcrystalline limestone and the sandstones are calcareous and fine to medium grained.

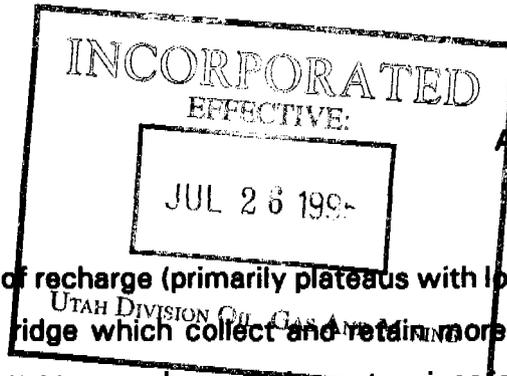
The Flagstaff Limestone forms much of the ridge top locations within and adjacent to the mine plan area. The limestone caps are relatively flat and low surface relief allowing larger quantities of water to infiltrate the surface soils. Water transmitting abilities of the Flagstaff are mainly due to solution channels and fractures (Danielson, et al., 1981). However, as discussed previously, nearly impermeable shale layers in the underlying North Horn and Price

River Formations cause much of the water percolating down from the Flagstaff to issue from the North Horn and Price River Formations as springs.

**Faults**—On a regional level, the ability of various formations to readily transmit water is primarily due to faulting and fracturing. However, as indicated in Chapter 6 of the Mining and Reclamation Plan, "faulting is relatively minor in the region and absent within the Mine Plan Area boundaries". "Folding is present only in very broad, gentle flexures, resulting in dip variations of less than 4 degrees." Therefore, the absence of large springs within the mine plan area can be attributed to the absence of faulting within the mine plan area. In general it has also been found that springs within the Blackhawk Formation are not fault related since shales tend to seal faults, preventing significant transmission of water through faults.

7 1-2 **Regional Groundwater Characteristics**

Waters recharging the groundwater system are primarily from snowmelt, with the quantity of recharge being highly variable from site to site as a result of topographic variation and variation in intake rates of the strata. Principal recharge areas for the groundwater system are the higher plateaus which receive higher precipitation and produce most of the runoff. The flatter slopes or low surface relief along ridge-tops of the higher plateau regions encourage infiltration. However, in general within the vicinity of the mine plan area the ridge-tops are narrow, providing less recharge area, and the canyon slopes are steep, inducing mostly runoff with less infiltration opportunity and therefore less recharge to the groundwater system. All of the geologic formations which outcrop in the area have relatively low primary permeabilities which further reduces the quantity of recharge to the groundwater system. According to Price and Arrow (1974) and the U.S. Geological Survey (1979), annual precipitation recharging the groundwater system is probably much less than 5 percent. Limited groundwater recharge also occurs along areas of outcrop for some of the more permeable strata at stratigraphically lower positions. However, in general these outcrop zones are narrow and located on steep slopes thereby limiting recharge potential.



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Groundwater movement is from areas of recharge (primarily plateaus with low surface relief or basins on the windward side of the ridge which collect and retain more snow) to areas of discharge, including springs and stream courses. In general, most springs issue either above or below the Blackhawk Formation except in areas where the Blackhawk is the exposed formation along ridge-tops. Springs issuing from the Price River, North Horn, or Flagstaff Formations are discharge areas for a perched aquifer system(s) within these formations. The perched aquifer condition results from the shale layers which impede vertical percolation forcing the groundwater to issue at a sandstone-shale interface as a spring. Groundwater discharging from the Blackhawk Formation either as springs or into stream courses is associated with the regional water table.

Within areas of the Wasatch Plateau-Book Cliffs region, it has been demonstrated that principal groundwater discharge areas for formations overlying the Blackhawk and for the Starpoint Sandstone beneath are fault or fracture related. A site specific lineament study conducted on a portion of the Huntington Creek drainage basin demonstrated that nearly all seeps or springs issuing from the Price River, North Horn, and Flagstaff Formations above the Blackhawk and from the Star Point beneath encountered during an intensive field survey were either fracture or fault related. Danielson et al. (1981) have indicated that within the formations of the Huntington Creek area, groundwater movement is primarily through fractures, along layer contacts, and through solution openings. The same formations exist within the Price River Coal mine plan area, therefore, groundwater movement is assumed to be similar.

Groundwater chemical quality is generally good in strata above the Blackhawk Formation within the Book Cliffs coal field. The total dissolved solids (TDS) concentrations of springs issuing from the Flagstaff or North Horn Formations can generally be expected to be less than 500 mg/l. Springs issuing from the Price River Formation can be expected to contain TDS concentrations of less than 700 to 800 mg/l. Waters percolating through the Blackhawk and Mancos Shale Formations quickly deteriorate in quality, with TDS concentrations of the Mancos Shale frequently exceeding 3000 mg/l.

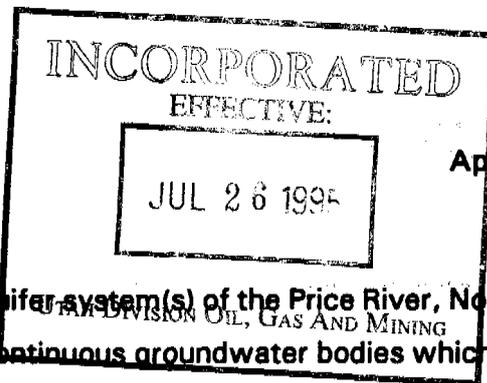
7.1-3 Mine Plan Area Aquifers, Seeps and Springs

Summarized within this section is a description of the mine plan area groundwater system under existing conditions. The existing hydrogeologic conditions within the mine plan area are highly complex as a result of the extensive historical disturbance to the hydrogeologic system that has occurred from prior mining activities. There have been 50 major coal mines which have historically mined coal within the limits of the plan area (some dating prior to 1900), 48 of which are now abandoned. All nine coal seams have been mined at one point or another with abandoned mine workings extending from the western to eastern edge of the mine plan area, a distance of some 14 miles (Adair, 1983). Therefore, the groundwater system has experienced disturbance in some form for over 85 years.

7.1-4 Description of Mine Plan Area Aquifers

The groundwater system of the Price River Coal mine plan area is highly complex due to the inter-bedded nature and presence of nearly impermeable shale layers of the Blackhawk, Price River, North Horn, and Flagstaff Limestone Formations. Additional complexity to existing aquifer systems is present as a result of the previously described extensive mining activities historically conducted within the mine plan area.

Three distinct aquifer systems (segregated by aquifer characteristics or locations) can be identified within the mine plan area; a perched aquifer system(s) in the Price River, North Horn, and Flagstaff Limestone Formations; the regional aquifer system probably originating in the Blackhawk Formation and extending into the underlying formations; and the alluvial aquifer systems, consisting of very narrow bands of alluvial deposits in the canyon bottoms of perennial streams within the mine plan area. Although the alluvial and regional aquifer systems are interconnected (the alluvium serving as either recharge or discharge area for the regional system), the alluvial aquifers can be distinguished from the aquifer system of the Blackhawk and underlying formations by differences in aquifer characteristics, the alluvial system in general being much more permeable than the surrounding formations.



**Perched Aquifer System** - The perched aquifer system(s) of the Price River, North Horn and Flagstaff Formations consists of "small, discontinuous groundwater bodies which receive natural recharge from local precipitation and discharge as small seeps and springs, many of which flow only seasonally (USGS, 1976)". The seeps and springs are located high in the watershed, generally at a sandstone-shale interface. The locations of springs will be discussed in a subsequent section of this report. As discussed previously, the shale layers within these formations impede groundwater movement perpendicular to the bedding plane, forcing the groundwater to move primarily parallel to the bedding plane and to issue as a seep or spring at the outcrop of the sandstone-shale interface. The shale layers are not absolutely impermeable; therefore, some leakage occurs through the shale layers, discharging limited quantities of water from the perched system to the underlying regional aquifer system.

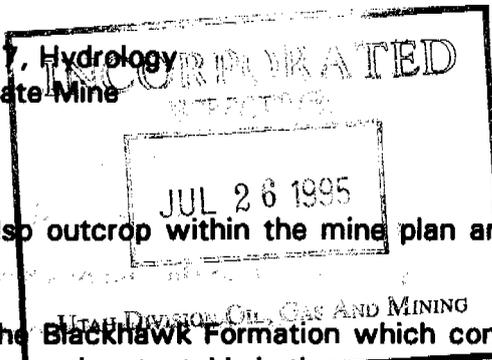
Principal recharge areas for the perched aquifer system(s) are the higher plateau regions or ridge-top locations which receive higher precipitation and regions which have flatter slopes, thereby encouraging more infiltration; or are local areas (or pockets) with higher than average snowfall accumulation. As indicated previously, the annual precipitation recharging the groundwater system is much less than 5 percent (Price and Arrow, 1974; and the USGS, 1979).

As indicated by the USGS (1976):

The rocks (including coal-bearing beds) that underlie the coal-lease area consist chiefly of poorly permeable shales, mudstones, siltstones, and fine grained sandstones that yield water slowly to wells and springs. A regional map showing general availability of groundwater in the upper Colorado River Basin (Price and Arrow, 1974) indicates that rocks in the coal-lease area are capable of yielding only 1 to 10 gal/min of water to individual wells. Most observed springs discharging from these same rocks in the general area of the coal lease yield less than 1 to 10 gal/min.

#### **7.1-5 Regional Aquifer System**

The regional aquifer system within the mine plan area is first encountered in the Blackhawk Formation and extends down into the Starpoint and Mancos Shale Formations,



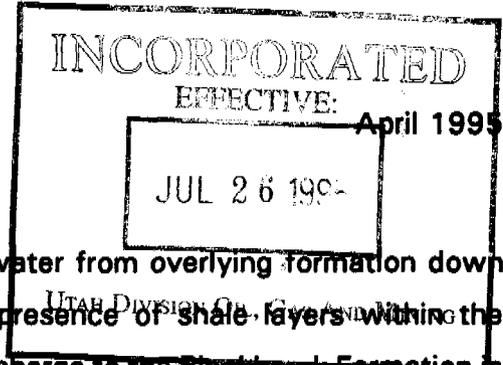
which also outcrop within the mine plan area (see Exhibit 7-1). According to the USGS (1976):

The Blackhawk Formation which contains the coal beds to be mined, is above the regional water table in the outcrop areas, but apparently extends beneath the regional water table to the north and between major canyons (where the altitude of the regional water table is the highest).

The rocks of the Blackhawk also consist chiefly of poorly permeable shales, mudstones, siltstones and fine grained sandstones. The low permeability of the Blackhawk Formation has been verified by testing three wells within the mine plan area (MC-205, MC-206, and MC-207, see Exhibit 7-2 for location). Aquifer testing was conducted by Golder Associates. Procedures used in the testing as outlined by Golder, are presented in Appendix 7-1. The drill logs as well as aquifer test results for the intervals over which each hole was tested are presented in Chapter 6 "Geology" of the Mining and Reclamation Plan. From the hydraulic conductivity values indicated on the logs for the various zones tested, a total transmissivity for each hole was determined by multiplying the thickness of each zone tested by the hydraulic conductivity of that zone and then summing the transmissivity values for all zones within each well. An average hydraulic conductivity was determined for holes MC-205 and MC-206 by dividing the total transmissivity by the total thickness of the tested zones. As illustrated by the results presented in Table 7-1, the entire Blackhawk Formation is extremely tight, having transmissivities on the order of 17 gpd/ft over a test zone of 808 feet and 65 gpd/ft over a test zone of 651 feet. Also as indicated on the log of hole MC-206, a test conducted over a 233-foot zone including multiple layers of coal yielded a hydraulic conductivity of only  $1.0 \times 10^{-6}$  cm/sec (transmissivity equal to 5 gpd/ft). Therefore, the coal is just as impermeable as other portions of the formation.

The Blackhawk, Starpoint and Mancos Shake Formations receive a limited quantity of recharge from exposed surfaces within the mine plan area. Direct recharge to strata of the Blackhawk and underlying formations is limited because of steep surface slopes and limited area of exposure throughout most of the mine plan area. Some additional infiltration opportunity is available for the Blackhawk Formation along several ridges where the Blackhawk Formation is exposed. However, in general these ridges are narrow, providing little

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Castle Gate Mine**

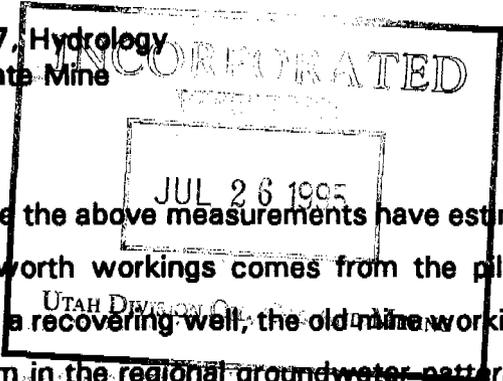


recharge area with lower relief. Movement of groundwater from overlying formation down into the Blackhawk is also quite limited due to the presence of shale layers within the Blackhawk and overlying formations. Consequently, recharge to the Blackhawk Formation is small, much less than the previously referenced 5 percent of annual precipitation as will be discussed in a subsequent section.

Discharge areas for the regional aquifer system within the mine plan area consist of a few springs (primarily alluvial in nature), water courses, and man-made sinks created by historical mining in the area. Springs issuing from the Blackhawk, Starpoint, and Mancos Shale Formations are illustrated on Exhibit 7-1 and will be discussed in more detail in a subsequent section. Principal water courses within the mine plan area include Spring Canyon Creek, Willow Creek, and the Price River. The major stream courses within the mine plan area have served as discharge zones for the regional system at one time or another. However, as demonstrated by the extremely low permeabilities of the Blackhawk Formation, recharge to the stream courses from the Blackhawk are very small. This is further illustrated by inflow into old abandoned mine workings.

The abandoned mine workings, as well as active workings which extend beneath the regional water table, currently serve as sinks in the regional aquifer system. However, seepage into these mines is extremely slow. Water inflow within Castle Gate No. 3 and No. 5 mines is outlined in Table 7-2. As indicated on Table 7-2, discharge into the No. 3 mine is only 33.4 gpm or 0.04 gpm/acre, and total discharge into the No. 5 mine is 3.5 gpm or 0.15 gpm/acre.

The abandoned mine workings are filling with water and, although those accumulations are substantial, the seepage rate into these mines is again very low. In fact, "substantial accumulations" have taken years to accumulate. For example, according to the USGS (1976) by 1976 the Kenilworth "A" Seam mine had accumulated an estimated 506 acre-feet of water over a 13 year period. This represents an average inflow of only 20 gpm. Measurements made by the Price River Coal placed the seepage rates into the old Kenilworth workings at 30 gpm. Seepage rates into the old Aberdeen, Utah Fuel No. 1, and Royal mines have been measured at 2.9 gpm, 12 gpm, and 30 gpm, respectively. \* Personnel of Price River Coal



who made the above measurements have estimated that over 90 percent of the water in the old Kenilworth workings comes from the pillared areas where subsidence has occurred. Similar to a recovering well, the old mine workings will continue to fill and serve as sinks until equilibrium in the regional groundwater pattern is again established.

**7.1-6 Alluvial Aquifer System**

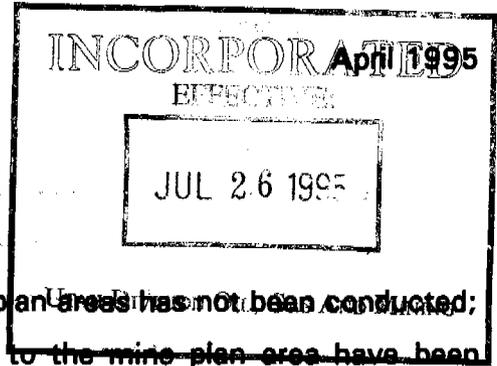
The principal alluvial aquifer systems within the mine plan area are located along the perennial stream courses of the Price River and Willow Creek; and the intermittent stream course of Spring Canyon Creek. Price and Wadell (1973) indicate that alluvial deposits along the Price River are quite permeable as indicated by the fact that wells developed into these alluvial deposits can be expected to yield flows of up to 500 gpm.

Primary recharge to alluvial deposits within the mine plan may be occurring from the regional aquifer system; however, due to the existence of abandoned or active mine workings which serve as sinks to the regional system and due to the low permeabilities of the Blackhawk and underlying formations, this recharge is small.

**7.1-7 Piezometric Contour**

It was indicated in the January 13, 1984, ACR comments review meeting with the State of Utah Division of Oil, Gas, and Mining, that due to the extensive historical disturbance from mining over the past 85 years, a piezometric contour would be indeterminable. Personnel of both DOGM and U.S. Office of Surface Mining (OSM) in attendance at the above indicated meeting agreed that a piezometric contour map would not be required.

Variations in groundwater level since 1980 are illustrated on Figure 7-2 for observation wells MC-205 (located in Sowbelly Canyon), MC-206 (located in Bear Canyon), and MC-207 (located in Crandall Canyon). The locations of these wells are illustrated on Exhibit 7-2.

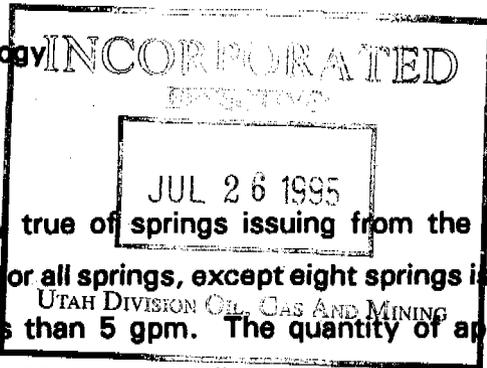


**7.1-8 Description of Mine Plan Area Seeps and Springs**

An onsite field survey of springs within the mine plan area has not been conducted; however, the locations of springs within and adjacent to the mine plan area have been determined using available information. Groundwater rights were inventoried to determine the locations, uses, and appropriated flow of all springs or wells for which a water right has been filed. Publications presenting hydrologic related information for the Book Cliffs coal field were reviewed and valuable insight as to the locations of prominent springs was gleaned from personnel of Price River Coal Company familiar with the mine plan area. The locations of all springs found during the above described survey are illustrated on Exhibit 7-1.

As illustrated on Exhibit 7-1, most springs issue from the Price River, North Horn and Flagstaff Limestone Formations overlying the Blackhawk Formation or from the Starpoint and Mancos Shale Formations which underlie the Blackhawk (most of which issue from channel bottoms). A total of sixty-one springs were located in the survey. Of these springs, forty-eight springs were found to be issuing from formations overlying the Blackhawk (six springs from the Flagstaff, sixteen springs from the North Horn, twenty-two springs from the Price River, and four springs from the Castlegate). Only three springs were located issuing from the Blackhawk, one of which is issuing from the portal on the downhill side of an abandoned mine (Section 1, T13S, R8E), and one of which is located near a ridge-top (Section 7, T13S, R10E) where the Blackhawk forms the surface formation along the ridge-top or recharge zone for this spring. Ten springs were located issuing from formations underlying the Blackhawk (two springs from the Starpoint and eight springs from the Mancos Shale). It is interesting to note that all of the springs issuing from the Mancos are located in channel bottoms and that the four springs issuing from the Mancos, located in each major canyon beginning near the town of Kenilworth and moving east, are located at approximately the same elevation and in an approximate line which parallels the strike of the formation.

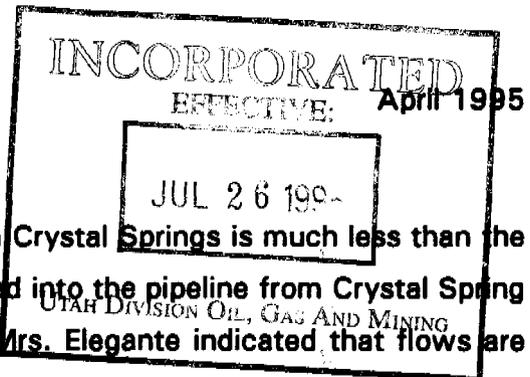
**Quantity** - According to the USGS (1976), most observed springs discharging from rocks within the "general area of the coal lease yield less than one to about 10 gpm".



This is particularly true of springs issuing from the Starpoint Sandstone. The quantity of appropriated flow for all springs, except eight springs issuing from the Star Point and overlying Formations, is less than 5 gpm. The quantity of appropriated flow for three of the eight springs is less than 10 gpm. The five remaining springs list an appropriated flow of 0.11 cfs (50 gpm) each. However, four of these springs (located in Section 32, T12S, R9E) are located near a narrow ridge-top and do not appear to have sufficient recharge area to support this flow. It is anticipated that the flows of these springs are more on the order of 0.011 cfs (less than 5 gpm). The only spring expected to be capable of having a flow greater than 10 gpm is the spring located in the SW1/4 of the NW1/4, Section 34, T12S, R10E, owned by the Bureau of Land Management and having an appropriated right of 0.11 cfs (50 gpm). As indicated by Mark Page, (Price Area Engineer, State Engineers Office), since most of these rights are diligence claims, the quantity of appropriated flow as listed on the water rights documents has probably never been measured and was estimated at the time of adjudication in 1956. Actual flows are probably less than the appropriated flow and many of the springs are probably intermittent, drying up during low flow periods of the year.

Appropriated flows for the four springs issuing from the Mancos Shale on the east side of the Price River is less than 5 gpm for each spring. However, appropriated flows for the 3 springs (Gravel Spring, Crystal Spring, and Goat Spring) issuing from the Mancos Shale west of Price River are 0.455 cfs (204 gpm), 1.0 cfs (449 gpm), and 0.15 cfs (67 gpm), respectively. Although the appropriated flow on Gravel Spring is 0.455 cfs, in April of 1975 a maximum sustained flow of only 50 gpm (0.11 cfs) was measured by personnel of the Price River Coal Company. Most rights associated with these springs are again diligence claims, and it is probable that flows were not measured from the springs when the rights were adjudicated and that the flows are overstated. Most water rights on Crystal and Goat Springs are held by the Elegante family. Water is piped from the springs through a small pipe of less than 2-inches in diameter to three homes which use the water for domestic needs and to water lawns. Mrs. Bertha Elegante (1983) indicated that when there is sufficient water, water from the springs is also used to irrigate a small orchard. However, the orchard has been abandoned for some 11 to 12 years due to insufficient water from the springs to supply

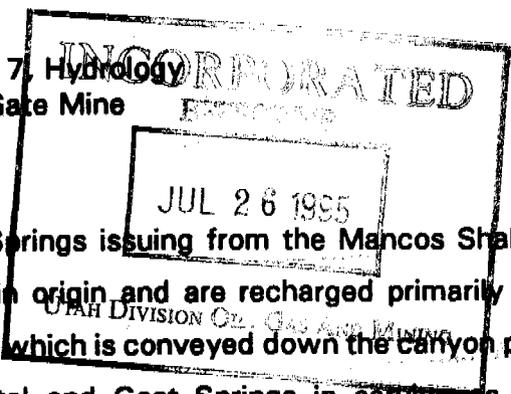
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the orchard. It is, therefore, expected that flow from Crystal Springs is much less than the 1.0 cfs of appropriated right. Excess flow not diverted into the pipeline from Crystal Spring was measured on May 12, 1983 at only 15 gpm. Mrs. Elegante indicated that flows are much higher this year (which would be expected due to the above normal water year that has occurred thus far). Considering the size of pipeline, the limited use of the water from Crystal Spring for only three homes, the excess flow of only 15 gpm, and the spring period with above normal precipitation when the measurement was taken (i.e. limited outdoor use of the water) actual flow from Crystal Spring is probably less than 50 to 100 gpm. The excess flow from Goat Spring was measured at 49 gpm.

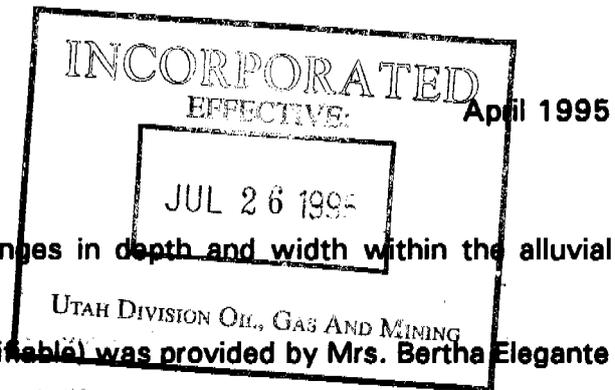
**Recharge** - Primary recharge to springs issuing from formations overlying the Blackhawk Formation (the perched aquifer system) occurs along the flatter slopes along ridge-tops of the higher plateau regions, which encourage infiltration and which receive higher precipitation. Within the mine plan area, ridge-tops are narrow (providing less recharge area) and canyon slopes are steep (inducing mostly runoff with little infiltration opportunity). In general, springs occur near local areas of recharge, primarily high in the watershed near local ridge-top recharge areas or downdip from pockets of above normal snowpack areas (i.e. in a canyon where wind patterns produce above normal accumulations of snowpack, generally on the east side of a ridge). As indicated previously, recharge to the groundwater system is probably much less than 5% of annual precipitation. According to Jeppsen et al. (1968), within the mine plan area normal annual precipitation varies from 12 to 20 inches. Therefore, normal annual recharge to the perched aquifer system is probably less than one inch.

Springs of the Blackhawk and underlying formations are discharge points for the regional and alluvial aquifer systems. Recharge to the regional aquifer system and associated springs results from leakage through shale layers of the perched aquifer system to the underlying regional system or from limited infiltration along the steep slopes of the outcrop areas of the formations. Since the regional aquifer system receives recharge due to leakage from the perched system, recharge to the regional system would be a fraction of the recharge to the perched system (again less than one inch).

  
Springs issuing from the Mancos Shale, both east and west of the Price River, are alluvial in origin and are recharged primarily from surface stream flow upstream from the springs, which is conveyed down the canyon primarily in the alluvium. Due to the importance of Crystal and Goat Springs in serving as a domestic supply, a site investigation was conducted on May 12, 1983, within the Spring Canyon area. Stream flow and conductivity measurements were taken from the mouth of Spring Canyon Creek to beyond the junction of the Left and Burnt Tree Forks, the results of which are presented on Exhibit 7-2.

As illustrated on Exhibit 7-3, the main stream disappears and reappears in the form of springs issuing from the channel bottom. At the junction of the two forks in Section 7, T13S, R9E, the total streamflow was measured at 220 gpm. Approximately two miles downstream at Measuring Station No. 8 the flow had reduced to 29 gpm. Just below Station No. 8, streamflow in Spring Canyon Creek was augmented by an additional 29 gpm from an easterly flowing tributary. Within one-half mile from the junction of the tributary with the creek, the surface streamflow had dropped to 11 gpm and then to a dry channel within approximately one mile. The stream was dry until Crystal Springs which was issuing from the bottom of the channel in Section 15, T13S, R9E. Within one-half mile downstream from Crystal Spring, the stream had again dried up and remained dry until Goat Spring which was issuing from the channel bottom near the eastern edge of Section 22, T13S, R9E.

Between Crystal Spring and Station No. 2 a slight degradation in water quality (as indexed by conductivity measurements) was noted. However, the conductivity measurements were surprisingly constant and low, indicating that little recharge is occurring from the Mancos Shale. There were no geologic features evident at the site that would indicate that the springs might be related and, as determined by prior mining activities, no significant faults have been identified within the mine plan area. The Mancos Shale surrounding the alluvial deposits of the creek bottom serves as an effective liner for waters moving within the alluvial deposits and probably prevents any significant loss of water to or inflow from the formation except water loss by transportation. Therefore, primary recharge to the springs issuing from the alluvium is from surface stream flow which infiltrates into the alluvium upstream from the



springs and resurfaces as springs due to changes in depth and width within the alluvial formation at the springs locations.

Additional clarification (although not verifiable) was provided by Mrs. Bertha Elegante (principal water rights holder Crystal and Goat Springs) as to the origin of waters from the springs. Mrs. Elegante indicated that since the principal mines upstream from the springs at Mutual, Rains, and Standardville have shut down, she has noticed a drop in the flows of Crystal and Goat Springs. Although flow records are not available on the springs for verification of the above statement, the old mines, when in operation, discharged water into Spring Canyon Creek above the springs, which would have supplied additional recharge for the down stream springs.

#### 7.1-9 Groundwater Quality

A water quality summary of data obtained from groundwater monitoring sites within the mine plan area is presented in Section 7.2 of this Chapter.

**Groundwater Use** - As indicated by the designated use on water rights, groundwater use within the mine plan area is limited primarily to stock watering. With the exception of one water right, all water rights for springs issuing from the Blackhawk and overlying formations have a designated use as stock watering. One water right for the spring issuing from the Price River Formation (SE1/4, NW1/4, Sec. 33, T1S, R9E) has a designated use of stock watering and domestic. However, considering the lack of access to this spring, it is doubtful that it has been used for domestic purposes.

Of the two springs issuing from the Starpoint Sandstone, both have appropriated rights of less than 5 gpm. The Starpoint Sandstone spring located in the NW1/4 of the SW1/4, Section 7, T13S, R10E lists the designated use as stock watering. The water right for the spring (controlled by Castle Gate) located in the NW1/4 of the SE1/4, Section 8, T13S, R9E lists the use as domestic.

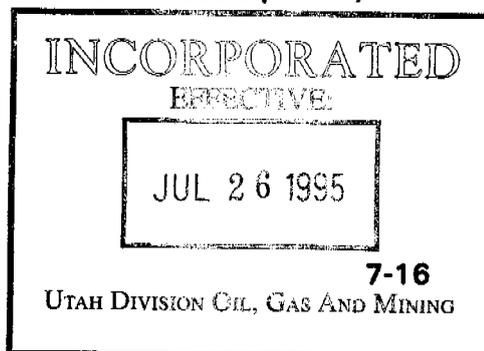
Water rights, listing use as industrial, are all controlled by Castle Gate and have a designated source as "underground water, tunnel".

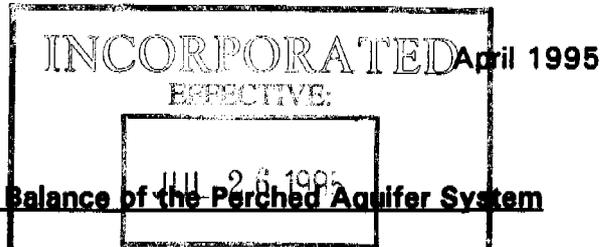
The seven springs issuing from the Mancos Shale supply water for stock watering, domestic, or irrigation purposes. The Castle Gate owns the first two springs located east of the Price River. The indicated use from water rights for the first three springs east of the Price River is stock watering and for the fourth spring (located in Section 18, T13S, R11E) is stock watering and domestic. West of the Price River, Gravel Spring is controlled by Castle Gate and supplies industrial makeup water for its No. 5 Mine. Crystal and Goat Springs supply the domestic needs for three homes and are used outside for watering lawns and when sufficient supply is available for irrigating a small orchard.

#### 7.1-10 Effects of Mining Operation on Groundwater

The hydrogeologic characteristics of the coal-bearing Blackhawk and overlying formations will effectively limit the extent of impacts to the hydrologic system by the mines of Castle Gate Coal Company. Therefore, impacts from the mines will be of local, as opposed to general regional, significance.

As indicated previously, the groundwater system has been extensively altered by previous mining within the mine plan area which has been conducted over the past 85 years. Over fifty major coal mines have been operated within the mine plan area, forty-eight of which are abandoned. All nine coal seams have been mined at one point or another, with abandoned works and pillared areas extending from the western edge to eastern edge of the mine plan area (14 miles). \* Therefore, the groundwater system has experienced disturbance of one form or another from extensive mine workings over the past 85 years. Again, due to the characteristics of the affected formations, impacts are of only local import to water resources within and immediately adjacent to the mine plan area. Potential impacts to the various aquifer systems and uses of these aquifer systems will be discussed separately in this section of this report.





**7.1-10(1) Impact Projections to the Hydrologic Balance of the Perched Aquifer System**

As evidenced by the locations of springs issuing high in the watershed from the Price River, North Horn, and Flagstaff Limestone Formations, a perched aquifer system (b) exists within these formations. Most springs issuing from these formations are separated by over 1500 feet of overburden from the underlying coal seams to be mined in the Blackhawk Formation. Therefore, impacts to the perched aquifer system are limited primarily to subsidence. According to the USGS (1976):

Land subsidence would ultimately occur above mined out areas, and subsidence would have some effect on local water resources. Because much of the coal-lease area has already been undermined, only part of the eventual subsidence would be attributable to the proposed mining operation.

According to Golder Associates (1983):

Subsidence of the surface is expected. It is our opinion, however, that because of the depth at which mining will take place averaging in excess of 1500 feet, coupled with the comparatively thin seam thickness (in the range of 12 feet), the surface subsidence will result in a relatively minimal amount of disturbance. Longwall mining techniques generally result in relatively controlled and non-catastrophic subsidence in response to coal extraction. As discussed in the application, the predominant main roof sandstone members overlying the coal seams by some 100 to 500 feet are expected to be supported on the siltstones and shales that form the immediate roof above the coal being mined. These siltstones and shales will break and cave behind the retreating wide longwall face. The broken caved material will swell to fill the void created by removal of the coal. The stronger upper sandstone members will then rest on and squeezing broken material without themselves breaking. The squeezing will continue until stable conditions are restored, by which time the predominant sandstones will have subsided slowly without breaking and will permit those overlying members and surface materials to subside correspondingly.

As indicated by Golder Associates, the upper limit of the fracturing and cracking above the subsided coal seam is expected to be less than 500 feet. Elastic deformation will occur within the overlying strata from the limit of the fracture zone to the surface. All of the springs of the perched aquifer system within the mine plan area issue at a location well above the predicted fracturing zone (which should be limited to the Blackhawk Formation). Since fracturing will not extend into the shales, which create the perched aquifer condition in the

formations overlying the Blackhawk Formation, perched waters in the overlying formations should be unaffected by mining.

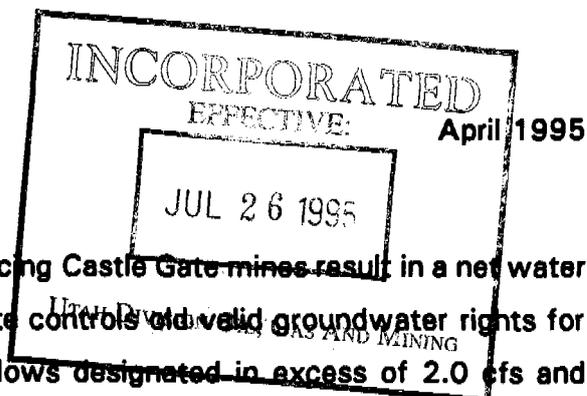
**7.1-10(2) Impact Projections to the Hydrologic Balance of the Regional Aquifer System**

The abandoned mine workings, as well as active workings currently serve as sinks in the regional aquifer system. However, seepage through the Blackhawk Formation into these mines is extremely slow, with a total water inflow in Castle Gate No. 3 and No. 5 mines of less than 50 gpm. As referenced previously, seepage rates into the abandoned mine workings of the Aberdeen, Utah Fuel No. 1, Kenilworth, and Royal mines have been measured at 2.9 gpm, 12 gpm, 30 gpm, and 30 gpm respectively. Seepage into the Adit No. 1 (Utah Fuel No. 1) Mine which is located in the A coal seam also includes seepage from the overlying abandoned mine workings in the C and D coal seams.

Water made within the active mines results in a net depletion of storage in the groundwater system. As indicated previously, recharge to the regional groundwater aquifer is expected to be much less than one inch. Recharge (expressed as a depth) to the abandoned mine workings for which inflow measurements have been made and to the Castle Gate No. 3 and No. 5 mines are presented in Table 7-3. Recharge to the abandoned workings varies from 0.46 of an inch at the Royal Mine to 0.08 of an inch in the Aberdeen Mine. Recharge to the No. 3 mine just after the mine was shut down was 1.02 inches and three months after shutdown had reduced to 0.70 of an inch and falling. As the mine face advances additional water is removed from storage to maintain a recharge rate into the mine in excess of natural recharge to the aquifer system.

As indicated by both the No. 3 Mine and the abandoned mines, as the Castle Gate mines are abandoned and face advance ceases, the mines will continue to fill and serve as sinks until equilibrium to the regional groundwater pattern is again established. As the face advance ceases, storage is depleted and inflow rates reduced until equilibrium between recharge and discharge rates.

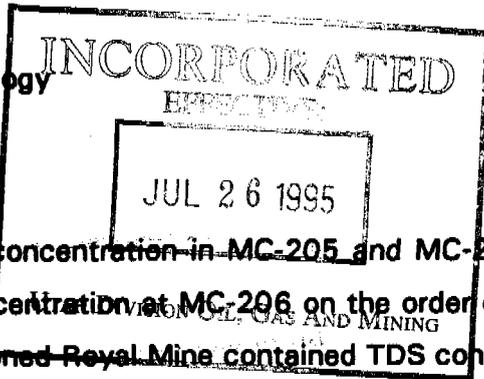
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It should be noted that although the advancing Castle Gate mines result in a net water depletion to the groundwater system, Castle Gate controls old valid groundwater rights for their underground workings with appropriated flows designated in excess of 2.0 cfs and priority dates of 1882 and 1925. It should also be noted that there apparently are no production water wells or springs located within the Blackhawk Formation that could be impacted by mining. Therefore, Castle Gate holds a valid right to deplete water from storage and to use that water for industrial purposes within the mine and the right to do so has existed since 1882 and 1925.

Once abandoned, the Castle Gate mines which lie below the regional water table will gradually fill until either equilibrium is established within the mines themselves without discharging directly to the surface (i.e. seepage rates into the mines are equal to seepage rates out of the mines to underlying formations) or until equilibrium is established between seepage rates into the mine and seepage out of the mine via an old portal or rock tunnel. Rock tunnels or portals located or intersecting mine working at low points within the mine, control the water levels to which the abandoned works will fill. Water levels in both the abandoned Aberdeen and Adit No. 1 (Utah Fuel No. 1) mines have reached a level of equilibrium as a result of rock tunnels which intersect the mines and serve as spillways or overflows for the underground reservoirs contained therein. Groundwater from the Adit No. 1 Mine discharges from the entrance of the tunnel or portal into the Price River, and groundwater from the Aberdeen Mine discharges into the unnamed wash which flows through the town of Kenilworth. In a similar fashion, the Castle Gate mines once abandoned will gradually fill until either an overflow point is established through an old portal located at a low point in the system or until equilibrium is established between inflow and outflow within the mine itself.

The TDS concentrations of waters contained in the abandoned mine workings do not appear to be significantly different from TDS concentrations of groundwater obtained from monitoring wells in the Blackhawk Formation. Thereby indicating (as indexed by TDS) little degradation in water quality within the abandoned mine workings, as summarized in Appendix 7-2. Data obtained from monitoring wells MC-205 (B-41), MC-206 (B-42), and MC-207 (B-43) indicate that TDS concentrations within the Blackhawk vary from 1120 to 1887 mg/l with

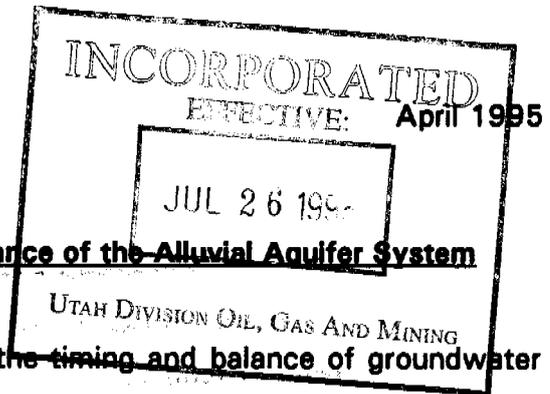


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an average TDS concentration in MC-205 and MC-207 on the order of 1200 mg/l and an average TDS concentration at MC-206 on the order of 1615 mg/l. Samples obtained from within the abandoned Royal Mine contained TDS concentrations on the order of 1200 mg/l. Discharge from the Adit No. 1 Mine on April 4, 1981, contained a TDS concentration of 1678 mg/l and water obtained from the abandoned Kenilworth Mine contained a TDS of 1210 mg/l on January 19, 1983. In a recent survey of Spring Canyon Creek, the specific conductance of water discharging from the abandoned mine portal located on the north section line of Section 12, T13S, R8E was measured at 1058 mhos/cm, assuming a TDS concentration of 690 mg/l. Thus, the mine water discharge in Spring Canyon was of a better quality than water in the creek (which measured at 1625 mhos/cm).

Primary impact to water quality of waters accumulating in the mines will result from oil and grease accumulations on the surface of mine waters. A heavy layer of oil and grease has accumulated on the water surface within the abandoned Royal Mine. Discharge of oil and grease from mine water to surface waters would result in a degradation of surface water quality until the oil and grease layer has been removed. Mitigation of this impact can easily be accomplished by siphoning off the oil and grease accumulations prior to discharging into surface waters.

Abandoned mine workings which intercept groundwater and discharge to the surface via a rock tunnel or old portal can result in a beneficial impact to the water quality of the Price River drainage system. Throughout much of the mine plan area, under pre-mining conditions most water currently intercepted by the mines would have passed through the Blackhawk into the underlying shales of the Mancos Formation prior to discharging into the stream courses. Interception of the groundwater by the mines short circuits this original groundwater pattern, resulting in groundwater discharge higher in the system before it can accumulate additional salts from the underlying shales. Therefore, overall water quality of the system is improved by the interception and discharge of groundwater higher in the system.



**7.1-10(3) Impact Projections to the Hydrologic Balance of the Alluvial Aquifer System**

Although it is true as described above that the timing and balance of groundwater discharge from the regional aquifer system courses has and will continue to be affected by mining, low permeabilities of the Blackhawk Formation and yield to abandoned and existing mines verify that this affect will be minor. Under full production, water inflow into the No. 3 and No. 5 active mines was only 50 gpm. As indicated previously, this inflow represents a depth of inflow across the contributing areas within the mines of approximately one inch which is in excess of predicted recharge of less than one inch. Therefore, much of this inflow is being removed from storage within the groundwater system and actual impact to flows into the stream courses is much less than 50 gpm. Under steady state conditions recharge to alluvial courses from the regional aquifer system would be equal to the recharge to the regional aquifer systems of much less than one inch. The ratio of the average depth of recharge to the abandoned mine works to annual recharge to the regional aquifer system is on the order of 0.28 inches. Applying this ratio of 0.28 to 1 to the Castle Gate No. 3 and No. 5 mines, actual impact to the Price River drainage system would be on the order of 14 gpm. From 37 years of historical streamflow records for the Price River near Heiner, Utah, the mean annual discharge of the Price River is 112 cfs. An impact of 14 gpm would represent a reduction in flow to the Price River of only 0.03 percent. Price River Coal Company holds some 1.7 cfs of water rights on the Price River which are more than adequate to mitigate any impact resulting from the minor reduction in yield discussed above.

In addition, Mark Page (Price Area Engineer, State Engineers Office) has indicated that they have noted no measurable impact to flows of the Price River as a result of the extensive mining which has occurred in the area.

Impacts to the stream courses from subsidence should be negligible. Although mining will occur beneath the Price River and Willow Creek, Castle Gate has indicated that areas underlying these perennial surface streams will not be pillared (subsided), thereby preventing damage to these surface sources from subsidence.

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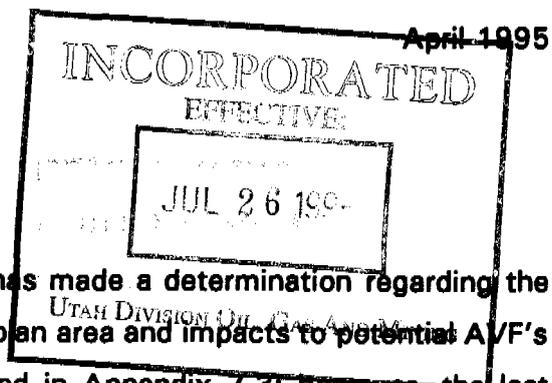
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According to the USGS (1976), "the proposed mine workings under Willow Creek and other canyons have been laid out in corridors where there is sufficient relatively impermeable rock material between the canyon floors and mine workings to prevent loss of streamflow to the mines". There appears to be a minimum cover of 600 to 800 feet between proposed mine workings extending beneath the river systems and the streambeds of the Price River and Willow Creek. This cover consists of interbedded shales, siltstones, coal, and tightly cemented sandstones, which if not disturbed will provide an effective barrier between the overlying stream and the mines. In order for water from the streams to infiltrate down into the underlying mines, the water must traverse the underlying layered rocks in a direction approximately perpendicular to the bedding planes. Due to the anisotropic nature of shales, permeabilities perpendicular to bedding can be as much as 10 times less than permeabilities parallel to bedding. Permeabilities parallel to bedding have been outlined previously in this text and have been shown to be extremely low. Therefore, movement of water from the streams to the mine perpendicular to bedding planes will be even more effectively inhibited than if flow were parallel to bedding.

Impacts to identified springs issuing from the alluvium in the canyons are also anticipated to be negligible. The field investigation conducted to determine the source of the major springs issuing from the channel bottoms of Spring Canyon showed that primary recharge for these springs is surface stream flows which recharge the alluvial deposits along the creeks upstream as higher elevations from the springs. Mining should have no impact on the surface runoff feeding these springs and, therefore, impact to these springs from mining is not anticipated. In fact, Mrs. Elegante, principal water rights holder on Crystal and Goat Springs (see Exhibit 7-1), indicated that spring flow has decreased since the mines that once discharged water to the creek upstream from the springs have been shut down. In other words, historically, mine water discharged into the stream provided additional recharge to the springs.



**7.1-10(4) Impacts to Alluvial Valley Floors (AVF)**

The Utah Division of Oil, Gas, and Mining has made a determination regarding the existence of an alluvial valley floor within the mine plan area and impacts to potential AVF's from mining. The entire DOGM report is presented in Appendix 7-3; however, the last paragraph is summarized below:

The Division has made the determination that present and future mining will not change the status or condition of the water resources, soils or geology relating to alluvial valley floors in or adjacent to the mine plan area. Mining will not interrupt or cause dilution of the existing groundwater or irrigation waters in a significant manner.

**7.1-10(5) Groundwater Intercepted by the Mine**

The inflow (50 gpm) into the No. 3 and No. 5 mines represents a depth of inflow across the contributing areas within the mines of approximately one inch which is in excess of predicted recharge of less than one inch. Therefore, much of this inflow is being removed from storage within the groundwater system and actual impact to flows into the stream courses is much less than one inch. Under steady state conditions recharge to alluvial courses from the regional aquifer system would be equal to recharge to the regional aquifer system of much less than one inch. Assuming recharge to the regional aquifer system to be equal to the average depth of recharge to the abandoned mine works, which is felt to be still somewhat higher, annual recharge to the regional aquifer system might be on the order of 0.28 inches.

At the end of 5 years, the mine will have extended beneath approximately 3500 acres of recharge area which at a rate of 0.28 inches per year would represent an impact to the Price River system of 51 gpm. Over the life of the mine, approximately 19,950 acres of recharge area will be disturbed which, at a rate of 0.28 inches per year, would represent an impact to the Price River drainage system on the order of 289 gpm (0.64 cfs). From 37 years of historical streamflow records from the Price River near Heiner, Utah, the mean annual discharge of the Price River is 112 cfs. The 5-year impact of 51 gpm would represent a

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reduction in flow to the mean annual discharge of the Price River of only 0.1 percent, and the life-of-mine impact of 288 gpm represent a reduction in flow to the mean annual discharge of the Price River of only 0.6 percent. Castle Gate holds some 1.7 cfs of water right on the Price River which are more than adequate to mitigate impacts resulting from the minor reduction in yield discussed above.

## **7.2 SURFACE WATER HYDROLOGY**

The Mine Plan Area is drained by two perennial streams, namely Willow Creek and the Price River. In addition, an intermittent stream, Spring Canyon, and numerous ephemeral streams dissect the landscape and provide channels from the drainage of precipitation. Flows in the perennial streams are definitely seasonal. Data from flows in these streams are presented in this chapter.

Monitoring of water quality and quantity has been underway since April, 1977, to assess the impacts of the mining activities on the regional hydrology. It is pertinent to note that springs that have been mapped prior to mining are still present and exhibit no observable reduction in discharge, indicating little or not impact as a result of mining activities. Section 7.3 provides a summary of monitoring data.

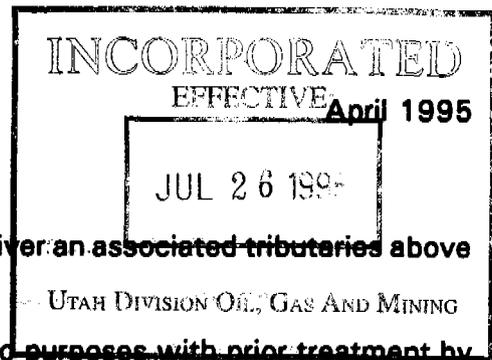
### **7.2-1 Existing Surface Water Regime**

The major perennial drainages included in this study are the Price River and Willow Creek. Minor drainages include Sowbelly Gulch, Spring Canyon, Sulfur Canyon, Hardscrabble and Ford Canyon creeks. Generally the minor drainages are ephemeral in nature.

Flows in the area range from 1.0 to 450 cfs in the Price River, 0 to 50 cfs in Spring Canyon Creek, and 0 to 242 cfs in Willow Creek. High flows in the ephemeral drainages are generally the direct result of thunderstorm activity.

Streams on or adjacent to the mine plan area fall into two use classes as defined by the Utah State Division of Health. The Price River and associated tributaries below the Price

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City water treatment plant are in the 3C class. The Price River, an associated tributaries above the treatment plan are classified as 1C and 3A waters

Waters classified as 1C are "protected for domestic purposes with prior treatment by complete treatment process". Class 3A waters are those "protected for cold water species of game fish and other cold water aquatic life, including the necessary aquatic organisms in their food chain". Standards for these waters are given in Table 7-4.

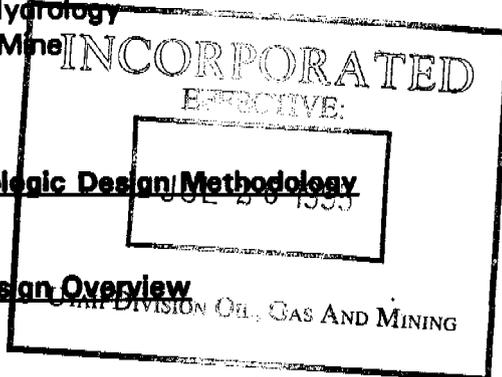
Waters classified as 3C (Table 7-5) are "protected for non-game fish and other aquatic life, including the necessary aquatic organisms in their food chain". Standards for this class will be determined on a case-by-case basis.

The watershed boundaries for the Price River (above the downstream limit of the mine complex), Willow Creek, Sowbelly Gulch, Spring Canyon, Bear Canyon, Crandall Canyon, Sulphur Canyon Creek, and Ford Creek are outlined on Exhibit 7-3 and 7-4. The watershed boundaries for Hardscrabble Canyon are outlined on Exhibit 3.3-3A. Tributary areas for the above indicated creeks are presented on Table 7-6.

The mean annual discharge of the Price River near Heiner from 37 years of records is 112 cfs (81,140 ac-ft/yr). Only two years of records are available for Willow Creek near its mouth (at Castle Gate). However, 19 years of available records for Willow Creek at the USGS gage near Castle Gate (see Exhibit 7-3) indicate the mean annual discharge to be on the order of 8.2 cfs (5,900 ac-ft/year). A comparison of the two years of overlapping records from the two gaging stations on Willow Creek indicates that at 8.1 cfs (5,900 ac-ft/year) at the upper site, an average discharge of approximately 9 cfs (6,5000 ac-ft/year) can be expected at the mouth of Willow Creek (an increase of approximately 10 percent between the two sites). Only three years of stream flow records are available at the USGS gage on Spring Canyon Creek. However, as indicated by these records, the yield from over 1,400 acres of drainage area is extremely low, averaging only 0.3 cfs (214 ac-ft/year). Stream flow records are not available for the non perennial tributary drainages in the area.

**7.2-2 Hydrologic Design Methodology**

**7.2-2(1) Design Overview**



This section presents the methodology used to perform hydrologic and hydraulic calculations for both operational and reclamation phases of all areas within the Castle Gate Mine complex. The hydrologic calculations explained below consist of the determination of runoff volume, peak runoff discharges, watershed sediment erosion, and sediment pond storage capacities. The methods used to perform hydraulic calculations to design temporary diversions, permanent stream channels and temporary spillways are discussed. Riprap and filter material design methods are also discussed.

**7.2-2(2) Runoff Calculations**

**7.2-2(2)A Operational Phase**

The watershed boundaries used to determine precipitation runoff within each canyon are shown on their respective existing drainage structures map (Sowbelly Canyon: 3.2-2B, Hardscrabble Canyon: 3.3-3A, Castle Gate Preparation Plant: 3.4-2, Adit No. 1: 3.5-2, Gravel Canyon: 3.6-2, and Crandall Canyon: 3.7-7). With the exception of Hardscrabble Canyon, the undisturbed watershed boundaries not shown on the above-mentioned exhibits are indicated on Exhibit 7-3. The Hardscrabble Canyon undisturbed watershed boundaries are all contained within Exhibit 3.3-3A. Each operational phase watershed is labeled according to basin, watershed and whether it is disturbed or undisturbed. The following basin codes are used in this labeling system: Sowbelly Canyon - SB, Hardscrabble Canyon - HC, Castle Gate Preparation Plant - CG, Adit No. 1 Canyon - AC, Gravel Canyon - GC, and Crandall Canyon - CC. The letters 'U' and 'D' are used to differentiate between predominately undisturbed and disturbed watersheds. Occasionally, watersheds are partitioned into subwatersheds, in which

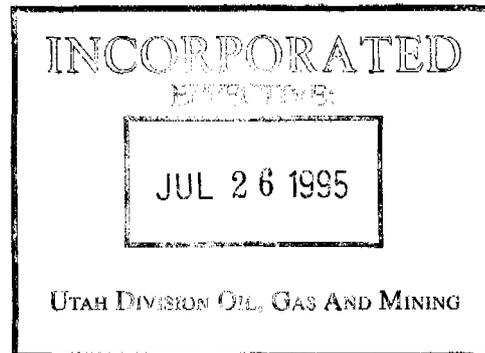
case the suffix letters A, B, C or D are used. For example, SBWS-D7A refers to Sowbelly Canyon operational disturbed Subwatershed No. 7A.

According to the U.S. Soil Conservation Service (1972), the algebraic and hydrologic relations between storm rainfall, soil moisture storage, and runoff can be expressed by the equations,

$$Q = \frac{(P-0.2S)^2}{P+0.8S} \tag{1}$$

and

$$S = \frac{1000}{CN} - 10 \tag{2}$$

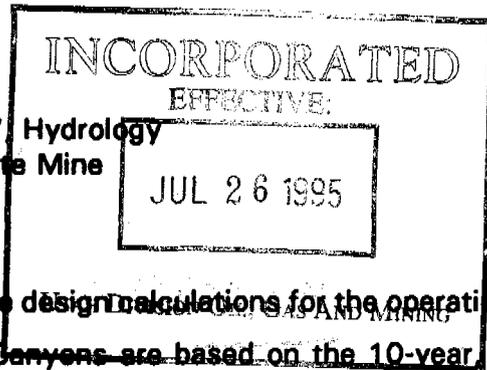


where, Q = direct runoff volume (inches)  
 S = watershed storage factor (inches)  
 P = rainfall depth (inches)  
 CN = runoff curve number (dimensionless)

It should be noted that (a) Equation (1) is valid only for  $P \geq 0.2S$  (otherwise  $Q = 0$ ), (b) Equation (2), as stated, is in inches, with the values of 1000 and 10 carrying the dimensions of inches, although metric conversions are possible, and (c) CN is only a convenient transformation of S to establish a scale of 0 to 100 and has no intrinsic meaning.

A curve number for each undisturbed area was chosen using professional judgement and the tabulated values presented in Figure 7-3, using approximate cover densities as reported in Chapter 9. Curve numbers ranging from 65 to 85 were obtained for the various undisturbed areas, assuming a hydrologic soil group of C. Soils in group C have a slow infiltration rate when wet, and are chiefly moderately deep, well-drained soils of moderately fine to moderately coarse texture (Barfield, 1981).

The curve number for disturbed areas was chosen from professional judgement and tabulated values presented by the U.S. Soil Conservation Service (1972). Accordingly, a value of 90 was used for all areas within the disturbed area boundary.

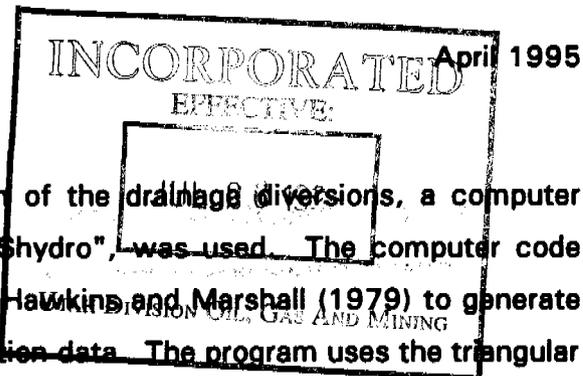


The design calculations for the operational phase diversion structures in Sowbelly and Crandall Canyons are based on the 10-year 24-hour storm event, in accordance with the 1987 Utah Division of Oil, Gas and Mining, Utah Coal Mining and Reclamation Regulatory Program. Similarly, the spillway design calculations for the Sowbelly Canyon sedimentation ponds (003, 004, 005) and Crandall Canyon Pond 014 were performed using the 25-year, 24-hour storm event. Since the submission of those designs, DOGM has changed the regulations regarding the design storm event. The 10-year 6-hour storm is now the required storm event for the design of temporary intermittent and perennial diversions (R645-301-742.323, DOGM, 1991). The 25-year 6-hour storm is now the required storm for the design of non-MSHA pond spillways, inflow and outflow structures (R645-301-742.223).

Recent modifications to several sedimentation ponds implemented the 25-year, 6-hour storm event requirement. Calculations performed during the pond modifications were compared to the peak flow and runoff volume from the previously required 24-hour storm. In all cases, the peak flow and runoff volume computed using the 6-hour storm duration were significantly less than from the 24-hour storm duration. Therefore, the original design calculations for drainage diversions and sedimentation pond spillways performed using the 24-hour storm duration are adequate.

Calculations for drainage structures in all the other areas within the Castle Gate Mine complex have been updated and are based on the 1991, R645 - Coal Mining Rules. The design storm event for temporary perennial and intermittent diversions is the 10-year 6-hour storm. Regardless of the size of the drainage area, none of the operational diversions were considered ephemeral. All diversions, except those in Sowbelly and Crandall Canyon, were evaluated based on the 10-year 6-hour event. Inflow channels, spillways, and spillway outslope channels for all the ponds except 003, 004, 005 and 014 were evaluated using the 25-year 6-hour storm flow. The 10-year 24-hour precipitation event was used to evaluate all operational ponds for capacity adequacy.

The precipitation values for the design storm events were obtained from precipitation-frequency maps for the state of Utah prepared by Miller et al., (1973). The precipitation data are presented in Table 7-7.



To determine a peak flow for the design of the drainage diversions, a computer program, referred to in the calculations as "SCShydro", was used. The computer code associated with that program was developed by Hawkins and Marshall (1979) to generate runoff hydrographs from watershed and precipitation data. The program uses the triangular unit hydrograph approach developed by the U.S. Soil Conservation Service (1972). To complete the input file for this program, a time of concentration was calculated for each watershed. The parameters for that calculation were determined based on measurements taken from 1" = 100' topographic maps developed from aerial surveys.

The triangular unit hydrograph is shown in Figure 7-4 along with a typical curvilinear hydrograph. The triangular hydrograph is characterized by its time to peak ( $T_p$ ), recession time ( $T_r$ ), time of base ( $T_b$ ), and the relations between these parameters (i.e.,  $T_r = 1.67T_p$ ;  $T_b = 2.67T_p$ ). Thus, from the geometry of a triangle, the incremental runoff ( $Q$ ) can be defined by the equation,

$$Q = \frac{(2.67T_p)(q_p)}{2} \tag{3}$$

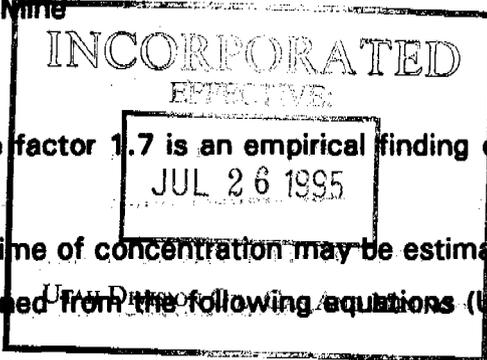
or

$$q_p = \frac{0.75(Q)}{T_p} \tag{4}$$

where,  $q_p$  = peak flow rate (dimensioned according to  $Q$  and  $T$ )

When  $Q$  is expressed in inches and  $T_p$  in hours,  $q_p$  will be in inches per hour. The flow at any time  $0 < t < T_p$  may be determined by simple linear proportioning of the triangular unit hydrograph. The time to peak is related to the familiar expression time of concentration ( $T_c$ ) by the equation,

$$T_c + t = 1.7T_p \tag{5}$$



in which the factor 1.7 is an empirical finding cited by the U.S. Soil Conservation Service (1972).

The time of concentration may be estimated by several formulas. For this report,  $T_c$  was determined from the following equations (U.S. Soil Conservation Service, 1972):

$$L = \frac{10.8(S+1)^{0.7}}{1900Y^{0.5}} \quad (6)$$

and

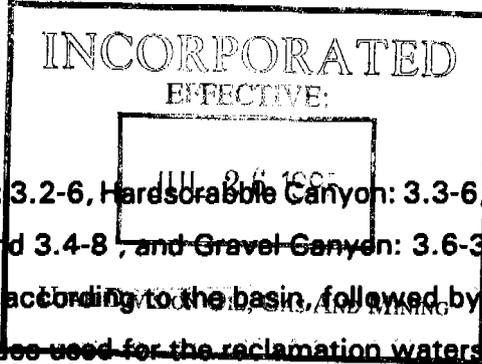
$$T_c = 1.67L \quad (7)$$

- where,
- L = watershed lag (hours). This is sometimes labeled as  $t_l$
  - l = hydraulic length of the watershed, or distance along the main channel to the watershed divide (feet)
  - S = watershed storage factor defined in Equation (2-2)
  - Y = average watershed slope (percent)
  - $T_c$  = time of concentration (hours)

Inflow hydrographs to and outflow hydrographs from the sedimentation ponds were developed using the hydrology and sedimentology model SEDIMOT II (Warner et al., 1980; Wilson et al., 1980). A more recent version of SEDIMOT II titled SEDCAD (version 3, 1992) was also used. These computer programs also use the rainfall-runoff function and triangular unit hydrograph of the U.S. Soil Conservation Service (1972).

**7.2-2(2)B Reclamation Phase**

Reclamation watershed boundaries used to determine the design precipitation runoff within in each canyon are shown on their respective reclamation topography exhibits (Sowbelly Canyon: 3.2-5, Hardscrabble Canyon: 3.3-5, Castle Gate Preparation Plant: 3.4-3, Adit No. 1: 3.5-3, Gravel Canyon: 3.6-3, and Crandall Canyon: 3.7-9). Boundaries which extend beyond the range of the 1" = 100' maps are contained on supplemental reclamation



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watershed boundary maps (Sowbelly Canyon: 3.2-6, Hardscrabble Canyon: 3.3-6, Castle Gate Preparation Plant: 3.4-8, Adit No. 1: 3.5-3 and 3.4-8, and Gravel Canyon: 3.6-3 and 3.4-8). Each reclamation phase watershed is labeled according to the basin, followed by a "RWS" to indicate a reclamation watershed. Basin codes used for the reclamation watershed labeling system are the same as those used for the operational phase system. The suffix of each watershed label has a number and either a "U" or "R" to differentiate between predominately undisturbed and reclaimed watersheds. Occasionally, watersheds are partitioned into subwatersheds, in which case the suffix letters A, B, C or D are used. For example, SBRWS-U9A refers to Sowbelly Canyon reclamation undisturbed subwatershed No. 9A.

The same precipitation-runoff relationships used to analyze operational diversions and ponds were used to design the permanent reclamation channels and temporary reclamation sediment ponds. The basis for curve numbers representing undisturbed watersheds remained unchanged, although a curve number of 80 was assigned to all future reclaimed areas within the disturbed area boundary. This value is commonly used for the time period after an area has been recently seeded and mulched, until the full establishment of vegetation.

The permanent perennial and intermittent reclamation channels were designed for the 100-year 6-hour storm event in accordance with R645-301-742.323. Permanent channels for ephemeral drainages were designed for a 10-year 6-hour event in accordance with R645-301-742.333. A drainage was considered ephemeral if (1) the watershed drainage area was less than 640 acres, (2) the drainage channel will flow only in direct response to precipitation or snow melt, and (3) the drainage channel will be above the local water table. All other reclamation channels were classified as intermittent/perennial.



**7.2-2(3) Sedimentation Ponds**

**7.2-2(3)A Pond Capacity**

The capacity of each operation and reclamation pond was determined based on runoff and sediment storage volumes. Both operation and reclamation ponds are designed to completely contain the 10-year 24-hour storm (R645-301-742.221.33, DOGM, 1991), and allow for adequate sediment storage capacity (R645-301-742.221.31, DOGM, 1991). In addition, R645-301-742.221.36 requires that sediment be periodically removed to maintain adequate volume for the design event. The runoff calculations for those watersheds contributing to each pond were performed as described in Section 7.2-2(2)A.

The annual sediment volumes entering the ponds were calculated using a modified version of the universal soil loss equation (Israelson et al., 1984). The modified universal soil loss equation is:

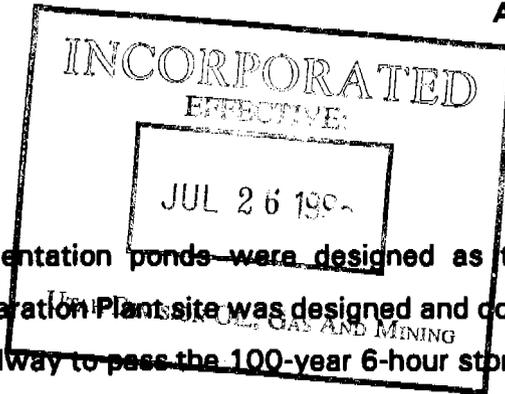
$$A = R \times K \times LS \times VM \quad (8)$$

where,

A	=	computed amount of soil loss per unit area for the time interval represented by factor R (tons per acre per year).
R	=	rainfall factor. R values for Utah presented by Israelson, et al. (1984).
K	=	soil erodibility factor. K values for Utah presented by Israelson et al. (1984). (tons per acre per year per unit of R).
LS	=	topographic factor based on length and steepness of slope. (dimensionless).
VM	=	erosion control factor based on vegetative, chemical, or mechanical measures. This factor is often referred to as $C_p$ . (dimensionless).

The maximum sediment storage volume and the 60% clean-out level was calculated for each pond.

**7.2-2(3)B Spillway Analysis**



All operation and reclamation sedimentation ponds were designed as temporary structures. Pond 013 at the Castle Gate Preparation Plant site was designed and constructed under MSHA guidelines which require the spillway to pass the 100-year 6-hour storm (R645-301-742.222, DOGM, 1991). The balance of the pond spillways are designed to pass the peak runoff from either the 25-year, 24-hour storm event (DOGM, 1987) or the 25-year, 6-hour storm event (DOGM, 1991), as explained in Section 7.2-2(1). Primary spillways are constructed as a riprapped channel or a corrugated metal pipe. The pipe spillways have either a hooded through-pipe or a rise-type drop inlet. A typical drop inlet design is presented in Figure 7-5. Several ponds also have riprapped trapezoidal emergency spillways.

The discharge capacity of the rise-type drop inlet was verified using methods described by Barfield et al. (1981). At low heads, the hydraulic capacity of the drop inlet spillway behaves as a weir. According to Barfield et al. (1981), the equation for weir-controlled flow is

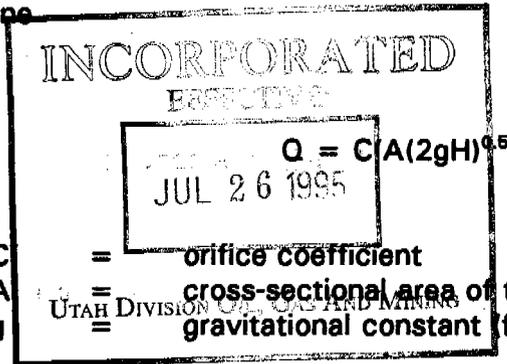
$$Q = CLH^{1.5} \tag{9}$$

where,

Q	=	discharge (cubic feet per second)
C	=	weir coefficient
L	=	length of the weir (feet)
H	=	depth of water above the weir crest (feet)

A weir coefficient of 3.1 was selected, since the structure will act as a broad-crested weir (Barfield et al., 1981). The length of the weir is equal to the circumference of the corrugated metal pipe riser.

As the depth of water increases above the riser in a riser-type drop inlet, the riser acts like an orifice. The equation for orifice flow is (Barfield et al., 1981)



(10)

where,

- C = orifice coefficient
- A = cross-sectional area of the inlet (square feet)
- g = gravitational constant (feet per second squared)

with all other parameters previously defined. A value of 0.60 was selected for the orifice coefficient based on guidelines presented by Barfield et al. (1981).

Pipe flow occurs when the head increases sufficiently to cause the outlet of the discharge pipe leading from the riser to flow full. The discharge capacity of the culverts under pipe flow conditions was determined using the equation,

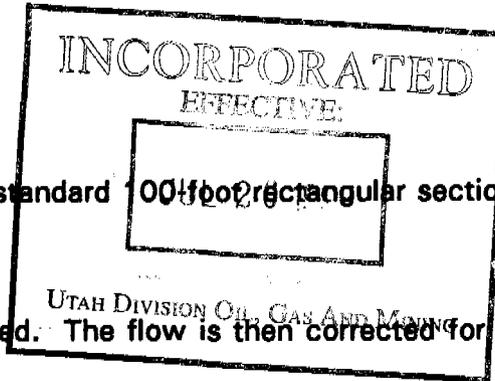
$$Q = A(2gH')^{0.5} / (1 + K_e + K_b + K_c L)^{0.5} \tag{11}$$

- where, H' = head on the pipe (feet)
- K<sub>e</sub> = entrance loss coefficient
- K<sub>b</sub> = bend loss coefficient
- K<sub>c</sub> = friction loss coefficient

with all other parameters previously defined. Values of 1.0 and 0.5 were used for K<sub>e</sub> and K<sub>b</sub>, respectively based on information provided by Barfield et al. (1981). The value for K<sub>c</sub> varies with the diameter of the pipe.

The discharge capacity of the riprapped overflow spillways was determined using a method developed by the U.S. Soil Conservation Service (1968) and expanded by Barfield et al. (1981) for broad-crested weirs. According to this methodology, the critical specific energy head (H<sub>sc</sub>) is determined for selected values of the energy head of water in the pond (H<sub>p</sub>) from Figure 7-6. The discharge capacity of the spillway is then calculated for the standard 100-foot wide rectangular section from the equation,

$$q_r = (0.544)(g^{0.5})(H_{sc}^{1.5})(100) \tag{12}$$



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where,  $q$  = discharge for standard 00-foot rectangular section (cubic feet per second)

with all other parameters previously defined. The flow is then corrected for a trapezoidal section using the equation,

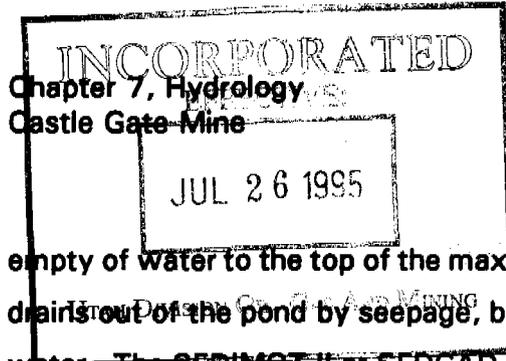
$$q = [(1.5b + zH_{oc})/150](q) \quad (13)$$

where,  $q$  = corrected discharge (cubic feet per second)  
 $b$  = bottom width of channel (feet)  
 $z$  = channel side slope (run over rise - dimensionless)

Spillway design calculations were performed using the hydrology and sedimentology model SEDIMOT II (Warner et al., 1980; Wilson et al, 1980) or SEDCAD (Warner, 1992). It should be noted that the sedimentology option of SEDIMOT II and SEDCAD was used during design only to permit routing of the hydrograph through the pond. However, since sediment contributions from the 25-year, 24-hour event (or the 25-year, 6-hour event) are not of concern in design of the pond spillways (only sediment yield from the 10-year, 24-hour and smaller storms is of regulatory concern), the sediment inputs to the model were suppressed. Thus, the output from the program indicates sediment concentrations of 0 milligrams per liter. Therefore, sediment yield outputs provided by SEDIMOT II and SEDCAD are meaningless.

It should also be noted that, although detention times shown on the SEDIMOT II output are relatively low (less than one hour), these times have no regulatory meaning for a 25-year event (i.e., regulatory concerns address the detention time only for the 10-year and smaller events). Again, the program was used primarily for its spillway design capabilities and not for dealing with the specifics of sediment yield and detention times from the 25-year design event.

The models SEDIMOT II (Warner et al., 1980; Wilson et al., 1980) and SEDCAD (Warner, 1992) assume that the pond is initially full of water to the elevation of the primary spillway when the storm event occurs. A more reasonable assumption is that the pond is



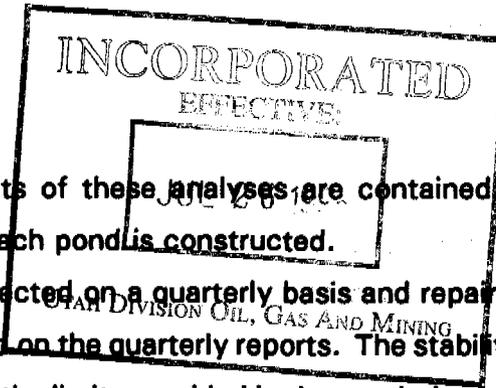
empty of water to the top of the maximum sediment storage level since storm water normally drains out of the pond by seepage, by opening the decant valve, or by manually pumping the water. The SEDIMOT II or SEDCAD models were initially run with the first assumption, and, if the spillway adequately passed the 25-year, 24-hour storm, additional analysis was not necessary. However, if adequate freeboard between the top of the embankment and maximum water level was not obtained, a stage-discharge curve was input to the SEDIMOT II or SEDCAD model which modeled the pond initially empty of water down to the maximum sediment level. The model was run a second time to determine the spillway performance and resulting freeboard.

Sedimentation ponds modified under the new regulations (DOGM, 1991) were designed assuming the ponds were full of water to the spillway flowline when the storm began.

**7.2-2(3)C Pond Construction and Maintenance**

The existing pond embankments were constructed in maximum lifts of 18 inches and compacted to 95% of the maximum density determined by the AASHTO T-99 compaction test. The inslopes and outslopes were designed not to exceed 1V:1.5H, except in areas where size is restricted. Embankments were keyed into the natural ground surface and unsuitable materials were excluded from the fill. Pond embankments were seeded, as described in Chapter 9, to reduce erosion.

Both the inslopes and the outslopes of the existing pond embankments were analyzed for long term stability. Since Pond 013 is a MSHA structure, the embankments were evaluated to assure that the static factor of safety exceeds 1.5 and the seismic factor of safety exceeds 1.2 (R645-301-533.100). All other sedimentation pond embankments (non-MSHA) were evaluated to address the requirements of R645-301-733.210 and R645-301-533.100 requiring a static factor of safety of 1.3 or greater. Horrocks and Carollo Engineers of American Fork, Utah analyzed the Pond 013 embankment using a computer software program based on the Modified Bishop method of slices. A second computer program, GEOSLOPE (Geocomp, 1988), was used by EarthFax Engineering, Inc. to evaluate the stability

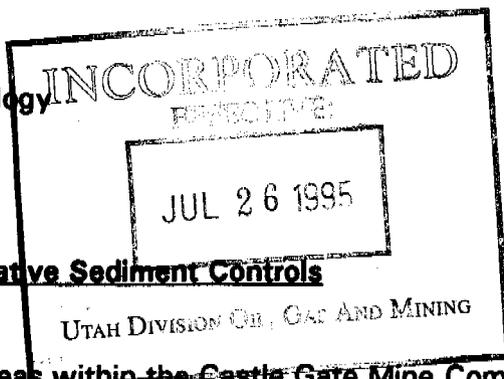


of the other pond embankments. Results of these analyses are contained in the permit section pertaining to the canyon where each pond is constructed.

Ponds and impoundments are inspected on a quarterly basis and repaired as needed. Any sign of weakness or failure is indicated on the quarterly reports. The stability of sediment pond embankments and slopes exceeding the limits provided in the regulations of Subchapter K, have been justified by their past performance. Existing slopes have demonstrated stability by past performance over the last ten or more years. Riprap at major inflow points is replaced if needed. Ponds and basins will be cleaned out when 60% of the design maximum sediment storage capacity has accumulated.

The sedimentation ponds will be drained of water, when necessary, to the maximum sediment storage elevation using either a permanently installed decant system, or a portable pump system. Prior to draining each pond, a sample of the water will be collected through the decant system and analyzed in accordance with R645-301-751 (DOGM, 1991). If the laboratory results are acceptable, the water will be drained from the pond to the decant flowline. If a visible higher sediment concentration is noticed during discharge, the process will be discontinued.

The portable pump system will be used on those ponds which are incised in the ground and are difficult to gravity drain with a permanent dewatering device. The intake on the pump system will be designed with a floatation device to collect water from the surface. It will also be designed with an oil skimmer to prevent the oil and trash floating on the surface from being drained. To insure that the sediment from the pond will not be drained, the intake will be designed with a base or frame sufficiently below the flowline.



**7.2-2(3)D Alternative Sediment Controls**

A list of areas ~~within the Castle Gate Mine Complex~~ which do not currently report to sediment ponds is presented in Table 7-8. The associated alternative sediment controls for each area are also identified in Table 7-8. Table 7-9 summarizes the alternative sediment control (ASC) areas and controls for the reclamation phases. Alternative sediment controls are used minimize the disturbance to the hydrologic balance. For the location of operational phase ASC areas, refer to the respective operational drainage structure exhibits for each canyon. For the location of initial reclamation ASC areas, refer to the appropriate reclamation topography exhibit for each canyon.

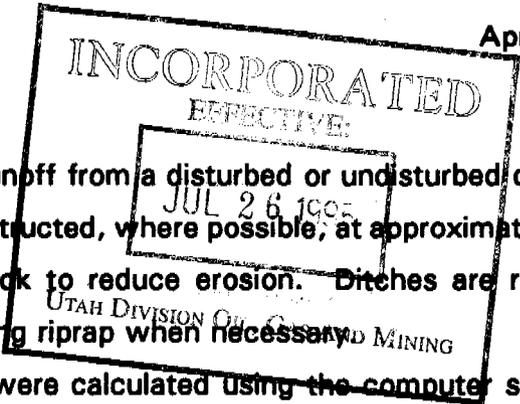
**7.2-2(4) Operational Diversion Structures**

**7.2-2(4)A Berms**

Earthen berms with a minimum height of 1.5 feet are constructed around the perimeter of many of the disturbed areas. The location of the berms are presented on the base map for each basin area (Exhibits 3.2-2A, 3.3-3, 3.4-2, 3.5-2, 3.6-2 and 3.7-7). The berms are labeled based on the basin and berm number. For example, GCB-2 represents Gravel Canyon berm No. 2. The berms are used to prevent random discharge from disturbed areas and to protect natural drainages or diversions. When berm construction is physically infeasible, staked straw dikes are used in their place. These berms are routinely maintained.

**7.2-2(4)B Drainage Diversions**

The location of diversion ditches are presented on the base map for each basin area (Exhibits 3.2-2A, 3.3-3, 3.4-2, 3.5-2, 3.6-2 and 3.7-7). The ditches are labeled based on the basin and diversion number. For example, CCD-5 represents Crandall Canyon diversion No. 5.



Diversions were designed to convey runoff from a disturbed or undisturbed drainage area. Grades on the diversion ditches are constructed, where possible, at approximately 5%. Some critical sections are riprapped with rock to reduce erosion. Ditches are routinely maintained by removing sediment and replacing riprap when necessary.

The ditch capacity and flow velocity were calculated using the computer software program called TRAP1 obtained from the Office of Surface Mining and outlined by Weider et al. (1983) or by FLOWMASTER I (Haested, 1990). Both of these programs are based on the Manning and continuity equations presented in Chow (1959):

$$V = \frac{1.486}{n} R^{0.67} S^{0.50} \quad (14)$$

and

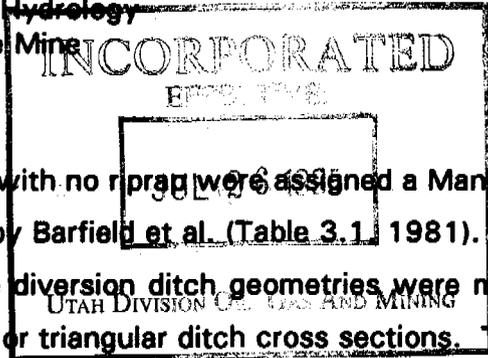
$$Q = AV \quad (15)$$

- where,
- V = velocity (feet per second)
  - R = hydraulic radius (feet)
  - S = hydraulic slope (feet per foot)
  - n = Manning's roughness coefficient
  - Q = discharge (cubic feet per second)
  - A = flow area (square feet)

Peak discharges for the undisturbed drainage areas were calculated as described in Section 7.2-2(1). A Manning's roughness coefficient was calculated based on the average size of the riprap in the diversion in accordance with the following formula presented by Barfield et al. (1981):

$$n = 0.0395 D_{50}^{1/6} \quad (16)$$

- where,
- n = Manning's roughness coefficient
  - $D_{50}$  = average riprap diameter in feet



Diversions with no riprap were assigned a Manning's roughness coefficient based on values presented by Barfield et al. (Table 3.1, 1981).

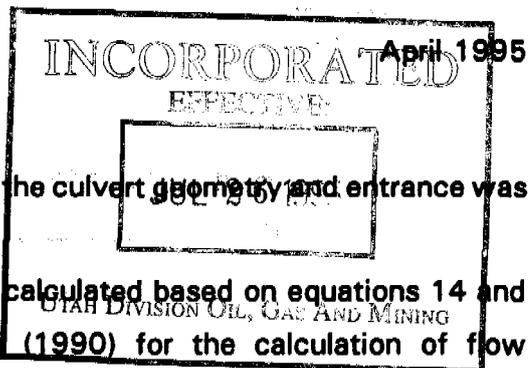
The diversion ditch geometries were measured in the field and approximated with trapezoidal or triangular ditch cross sections. The maximum and minimum hydraulic slopes of each ditch was either measured in the field or approximated from the topographic base maps (scale: 1" = 100', or 1" = 200').

With the values of peak design discharge, bottom width, side slopes, and Manning's coefficient, two trials were performed using FLOWMASTER I to calculate maximum flow depth and maximum flow velocity. For the first trial, the minimum channel slope was entered into the interactive computer program and a maximum flow depth calculated. Adequate freeboard was added to the maximum flow depth resulting in the minimum required diversion depth. In the second trial, the maximum channel slope was entered, yielding the maximum velocity to be used for riprap design. The adequacy of the existing riprap was then verified by following the procedure described in Section 7.2-2(6).

#### **7.2-2(4)C Culverts**

The location of operational diversion culverts are presented on the base map for each basin area (Exhibits 3.2-2A, 3.3-3, 3.4-2, 3.5-2, 3.6-2 and 3.7-7). The culverts are labeled based on the basin and culvert number. For example, CCC-5 represents Crandall Canyon culvert No. 5. The location, size, and slope of each culvert were verified in the field.

Peak discharges for the 10-year 24-hour storm event or the 10-year 6-hour storm event were calculated as described in Section 7.2-2(1). The adequacy of each culvert was determined using nomographs prepared by the U.S. Department of Transportation (1977). These nomographs for circular and pipe-arch culverts with inlet control are presented in Figures 7-7 and 7-8, respectively. Based on the known culvert size, entrance type, and peak discharge, the headwater depth/diameter ratio was determined from the nomograph. If this value was 1.0 or less, the culvert was considered adequate to pass the design discharge rate.



If the ratio was greater than 1.0, a closer inspection of the culvert geometry and entrance was necessary.

Exit velocities from each circular culvert were calculated based on equations 14 and 15. A computer program developed by Suchoski (1990) for the calculation of flow characteristics in circular channel sections was used to determine the normal depth and flow velocity in each culvert. Flowmaster I (Haested, 1990) was also used for culvert velocity calculations. A roughness coefficient of 0.024 was used for the calculations, which can be considered typical value for corrugated metal pipe (Chow, 1959).

The exit velocities and normal depth of flow for pipe arch culverts were determined by methods defined by the American Iron and Steel Institute (1983). Hydraulic characteristics were initially determined based on full flow conditions. Figure 7-9 was used to calculate the hydraulic characteristics at the desired peak discharge rate.

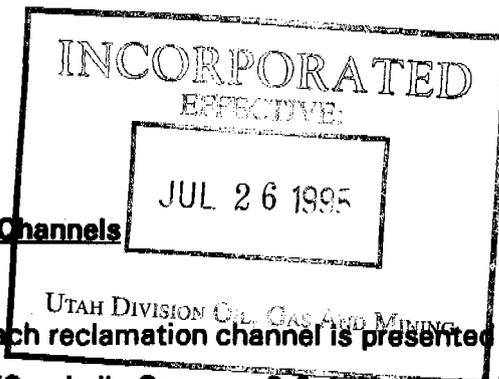
The adequacy of the existing riprap at each culvert outlet was determined using the methods defined in Section 7.2-2(6).

#### **7.2-2(5) Reclamation Structures**

##### **7.2-2(5)A Culverts**

The locations of permanent reclamation culverts are shown on the reclamation topography map for each canyon (Sowbelly Canyon: 3.2-4, Hardscrabble Canyon: 3.3-5, Castle Gate Preparation Plant: 3.4-3, Adit No. 1: 3.5-3, Gravel Canyon: 3.6-3, and Crandall Canyon: 3.7-9). The storm event used to design the reclamation culverts reflect the reclamation precipitation-runoff criteria discussed in Section 7.2-2(2)B. The culvert design procedures discussed in Section 7.2-2(4)C were used to design the reclamation culverts.

**7.2-2(5)B Reclamation Channels**



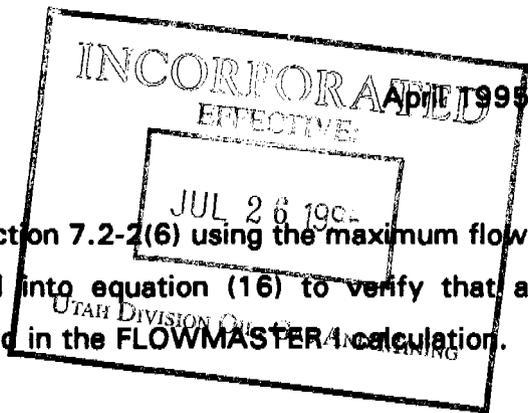
The location of each reclamation channel is presented on the reclamation topography map for each basin area (Sowbelly Canyon: 3.2-4, Hardscrabble Canyon: 3.3-5, Castle Gate Preparation Plant: 3.4-3, Adit No. 1: 3.5-3, Gravel Canyon: 3.6-3, and Crandall Canyon: 3.7-9). The reclamation channels are labeled based on the basin and watershed number. In several instances, channels are subdivided into sub-reaches for the purposes of riprap design. For example, HCRD-4A represents Hardscrabble Canyon Reclamation Diversion channel No. 4, reach A.

The reclamation channels were designed to approximate the geometry of the natural stream channels. The natural channel sections were measured in the field and approximated with a trapezoidal cross section. Except for existing diversions which will also be used during reclamation, the reclamation channels were designed with 3H:1V side slopes to provide channel stability. The hydraulic slope of each channel was measured from the postmining reclamation topography exhibits (scale: 1" = 100').

Curve numbers for the undisturbed drainage areas were estimated from vegetation data presented in Chapter 9, and by field observations. A curve number of 80 was assumed for the reclaimed areas.

Peak discharges for the drainage areas were calculated as described in Section 7.2-2(2)B. The 100-year, 6-hour storm event was used to determine the peak flow for each channel receiving perennial or intermittent flow, per R645-301-742.323. The 10-year, 6-hour storm event was used to determine the peak flow for permanent ephemeral channels, per R645-301-742.333.

FLOWMASTER I was utilized to size the reclamation channels by following procedures similar to those discussed in Section 7.2-2(4)B. The channel bottom width, side slopes, assumed Manning's coefficient, and peak discharge were input into FLOWMASTER for each channel. The maximum flow depth was calculated using the minimum channel slope, and the maximum velocity was determined using the maximum channel slope. One foot of freeboard was added to the maximum flow depth to determine the minimum reclamation channel depth.

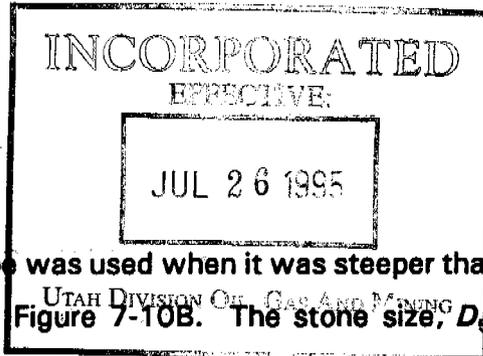


Once the riprap was designed in accordance with Section 7.2-2(6) using the maximum flow velocity, the average stone diameter was inserted into equation (16) to verify that a reasonable Manning's roughness coefficient was used in the FLOWMASTER calculation.

**7.2-2(6) Riprap Protection**

The use of riprap to line drainage ditches, culvert outlets, channel diversions and spillways is required when flow velocities exceed approximately three to five feet per second, depending on the nature of the soil and vegetation on the bottom of the channel (Barfield, et al., 1981). The size distribution and thickness of the riprap required to resist the erosive forces of the water is determined by first calculating the median riprap size, commonly referred to as the  $D_{50}$ . The  $D_{50}$  is the particle size for which 50% of the material by weight passes a screen with square openings whose lengths correspond to the particle size. Two methods were used to calculate the  $D_{50}$  of the riprap. The first method, developed by Searcy (1967) and later used by the U.S. Department of Transportation (1978), was used to size riprap for all operation phase hydraulic structures. In addition, this procedure was followed to design riprap for all ephemeral reclamation channels and for intermittent reclamation channels with longitudinal slopes less than 10%. The second method, developed by Simons, Li and Associates (1982) was used to size the  $D_{50}$  of the riprap for intermittent reclamation channels whose longitudinal channel slopes are equal to or greater than 10%. The Simons, Li and Associates method (1982) was not used to design the riprap for ephemeral channels with slopes greater than 10%, since the graphs associated with that method are not applicable to the low design flows typical of the ephemeral channels.

The method of Searcy (1982) utilizes Figures 7-10A and 7-10B to determine the  $D_{50}$  of channel or spillway riprap. The average size of stone,  $k$ , is determined by a trial-and-error method which consists of first estimating an average stone size. Using the flow depth,  $d$ , associated with flow through the steepest reach of the channel, a ratio of  $k/d$  is calculated. The mean velocity,  $V_m$ , of the water must then be converted to the velocity against the stone by the use of Figure 7-10A. Knowing the velocity against the stone,  $V_s$ , and the channel side



slope, (the hydraulic slope was used when it was steeper than the side slopes), the stone size can be determined from Figure 7-10B. The stone size,  $D_{50}$ , from Figure 7-10B is the 50 percent (median) size, by weight, of a well-graded mass of stone with a unit weight of 165 pounds per cubic foot. If the stone size from Figure 7-10B agrees with the assumed stone size, it is correct. If not, the procedure is repeated until agreement is achieved.

The method of Simons, Li and Associates (1982) was developed specifically for steep slope conditions. The procedure utilizes various graphs developed from relationships presented by Bathurst (1979) between the flow velocity, flow depth, and the flow resistance in mountain rivers. The graphs are presented as Figures 7-11A through 7-11E. These graphs relate maximum flow and channel slope for a given trapezoidal channel of specified bottom width and 2H:1V side slopes to a median riprap size ( $D_{50}$ ). Although Simons, Li and Associates (1982) recommends using 2:1 sides slopes for mining channels, Castle Gate has chosen to design the reclamation channels with 3:1 side slopes for channel stability and ease of construction. Since the flow area of a trapezoidal channel of specified bottom width and depth with side slopes of 3:1 is larger than the area of a channel with similar bottom width and depth with 2:1 side slopes, the velocity will be less in the channel with 3:1 side slopes. Consequently, the use of Figures 7-11A through 7-11E, developed for channels with 2:1 side slopes, yields conservative results when applied to channels with 3:1 side slopes.

To determine the  $D_{50}$  riprap size of a given reach of a channel, the appropriate graph corresponding to the channel bottom width must be selected. For bottom widths other than the ones the graphs were prepared for, Simons, Li and Associates (1982) allows linear interpolation of the results from the graphs bracketing the desired bottom width. Using the maximum flow rate, in cubic feet per second, and the channel slope (interpolating as necessary), the  $D_{50}$  can be determined from the appropriate graph. Table 7-10 is then used to select the design  $D_{50}$  of the riprap.

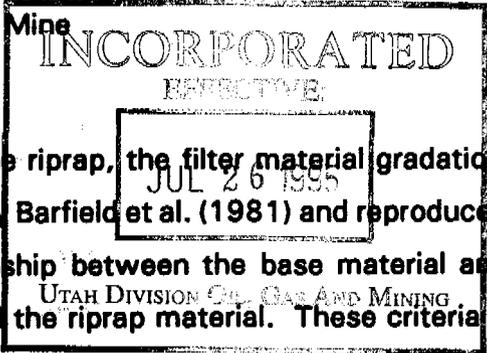
A detailed riprap gradation design for the reclamation channels is not presented in the permit documents, but will be prepared when the filter blanket gradation is designed. The commitment to prepare a filter blanket design is discussed below. The riprap gradation for the ephemeral channels and the mild slope sections of the intermittent channels will be

engineered based on methods presented in Barfield, et al. (1981). The procedure presented by Simons, Li and Associates (1982) will be used to design the riprap gradation for steep-slope intermittent reclamation channels.

Where the method of Searcy (1967) was used to calculate the riprap  $D_{50}$ , the thickness of the riprap was assumed equal to two times the  $D_{50}$ . If the method of Simons, Li and Associates (1982) was used to calculate the  $D_{50}$ , a riprap thickness equal to 1.25 times the  $D_{50}$  was assumed. To determine the width of riprap placement in a channel, the transverse length across the channel along the proposed ground surface was calculated. This length is referred to as the perimeter. The riprap volume is equal to the length of the channel reach multiplied by the perimeter, multiplied by the thickness (in feet). The weight of the riprap materials associated with the reclamation plan for each canyon was calculated assuming an in-place riprap density of 140 pounds per cubic foot.

Usually it is necessary to place a filter layer between the riprap and the underlying soil to prevent the migration of the soil out through the riprap (Simons, Li and Associates, 1982). The reclamation filter blanket will consist of a properly graded coarse-grained soil; a synthetic fabric will not be used. A detailed filter gradation design has not been prepared, since adequate soil samples were not available at the time these documents were compiled. Castle Gate is committed to preparing a detailed design for both the riprap and filter blanket gradations for the reclamation channels. In each canyon, soil samples will be taken once the reclamation grading has progressed sufficiently to expose the base of the reclamation channels. A sieve analysis of the soil samples will be performed in accordance with the procedures delineated in ASTM D117 (Materials Finer than the #200 Sieve in Mineral Aggregate by Washing) and ASTM D136 (Sieve Analysis of Fine and Coarse Aggregates). Using the test results, the filter gradation designs for each canyon will be prepared as described below and then submitted, along with the riprap gradation designs, to the Division of Oil, Gas and Mining for approval. Submittal will occur prior to delivery of filter materials to the canyon undergoing reclamation.

According to Barfield et al. (1981), a filter material is required when the ratio of the  $D_{50}$  of the riprap to the  $D_{50}$  of the underlying soil exceeds 40. Regardless of the method used



to design the riprap, the filter material gradation will be engineered using the three criteria presented by Barfield et al. (1981) and reproduced below. The following criteria evaluates the size relationship between the base material and the filter material and between the filter material and the riprap material. These criteria are:

-----  
Criteria 1  
-----

$$\frac{D_{50}(\text{filter})}{D_{50}(\text{base})} < 40 \quad \text{also} \quad \frac{D_{50}(\text{riprap})}{D_{50}(\text{filter})} < 40 \quad (17)$$

-----  
Criteria 2  
-----

$$5 < \frac{D_{15}(\text{filter})}{D_{15}(\text{base})} < 40 \quad \text{also} \quad 5 < \frac{D_{15}(\text{riprap})}{D_{15}(\text{base})} < 40 \quad (18)$$

-----  
Criteria 3  
-----

$$\frac{D_{15}(\text{filter})}{D_{85}(\text{base})} < 5 \quad \text{also} \quad \frac{D_{15}(\text{riprap})}{D_{85}(\text{filter})} < 5 \quad (19)$$

where:  $D_x$  = Diameter corresponding to the "x" percentage of soil passing through a specific screen, by weight. The screen has square openings which have lengths equal to the particle's "diameter."

The filter material which meets these criteria will minimize the possibility of piping under the riprap.

For reclamation volume calculations, the thickness of all the filter materials is set at one half the thickness of the riprap, but not less than 6 inches (Barfield et al., 1981). The width of filter placement in each channel is based on the perimeter of the channel, as defined above. Again, the volume of the filter blanket is equal to the length of the channel reach multiplied by the perimeter multiplied by the thickness (in feet). The weight of the filter materials associated with the reclamation plan for each canyon was calculated assuming an in-place filter density of 130 pounds per cubic foot.

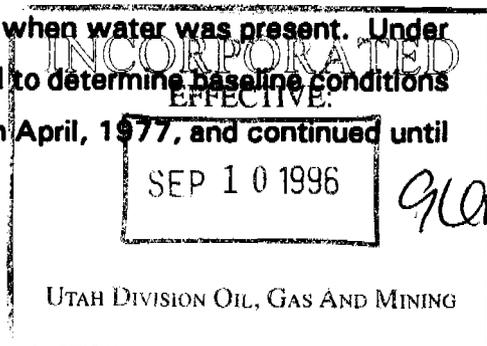
### 7.3 SUMMARY OF WATER MONITORING DATA

In 1977, in response to regulations set forth by both the DOGM and the OSM, Price River Coal Company (then Braztah Corporation now Castle Gate Coal Company) retained Vaughn Hansen Associates to design and conduct a water monitoring program for those areas which could be impacted by the Price River Coal Company mining operations.

The overall objective of the program was to allow determination of impacts on the surface and groundwater regimes in the Price River mine plan area.

**Scope** - The scope of this report is to describe the existing surface and groundwater hydrologic conditions of the Price River mine plan and adjacent areas. The report will cover the following topics: Methodology, Groundwater Resources, Surface Water Resources, and Impact of Mining Operations.

**Methodology** - Following promulgation of the DOGM and OSM regulations, Price River Coal Company (then Braztah Corporation now Castle Gate Company) had Vaughn Hansen Associates set up a twelve (12) station monitoring programs to determine baseline conditions (see Figure 7-12). These stations were monitored monthly when water was present. Under this initial program, forty-one (41) parameters were sampled to determine baseline conditions (see Table 7-11). This phase of the program was initiated in April, 1977, and continued until May, 1978.



As the study continued and federal and state requirements changed, the original program was modified in June, 1978 to meet the new requirements. Several modifications were also made to the monitoring program based on the data collected at that time. Review of the data showed that nineteen (19) stations were required to monitor the mine plan area (see Figure 7-13). Ten of the stations were monitored every other month and nine (9) were monitored twice per year. A review of the data showed that several parameters (aluminum, ammonia, boron, carbonate, chemical oxygen demand, copper, hydroxide, mercury, total phosphate, silica, total alkalinity, total hardness and turbidity) varied very little or were consistently below levels of concern as defined by state water quality standards. These parameters were dropped from the routine analysis and included in a system of spot checks. On a random basis, these parameters were analyzed to provide sporadic checks on parameter levels. Table 7-12 lists the water quality parameters analyzed.

In 1979, DOGM published guidelines "Proposed Surface and Groundwater Monitoring Guidelines", delineating the level of detail required in monitoring programs under the interim regulations. In the summer of 1979, Price River Coal Company (then Braztah Corporation now Castle Gate Coal Company) submitted a monitoring program based on the two previous years' data. Collection of water quality samples occurred at a bi-monthly frequency for surface water stations and springs. Bi-annual monitoring of groundwater wells was conducted in the spring and fall. The parameters analyzed are listed in Table 7-13. The sample stations remained unchanged through December of 1979.

Early in 1980, several monitoring stations were added to the program. Surface water station B-12 allowed for the determination of impacts above and below facilities in Hardscrabble and B-25 and B-26 allowed the determination of impacts proposed for Crandall Canyon. These stations and all those monitored from January 1980 to the present are shown on Figure 7-14.

Late in 1995, the Division raised a concern regarding the sampling of B-22 prior to November 1995. The Division's concerns are based on the fact that various places within Crandall Canyon have been called B-22 in the past and may have been sampled as such and not representative of water coming from B-22, i.e. a well near the Crandall Canyon shafts, one-half mile above Highway 50 & 6, and the culvert at the canyon mouth.

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With this in mind, and not to compare past data with future data, yet still tie future data to past, Castle Gate Coal Company established a new site to monitor the flow emanating from the alluvium within the stream channel and number it as B-22-1. The sampling point will vary depending on the location of the groundwater emanating from the alluvium within the streambed. During late spring and early summer, flow may emanate further up canyon within the stream channel with flow emanating further down the channel during drier periods.

Table 7-14 lists each of the monitoring stations with its townships, ranges, and section coordinates.

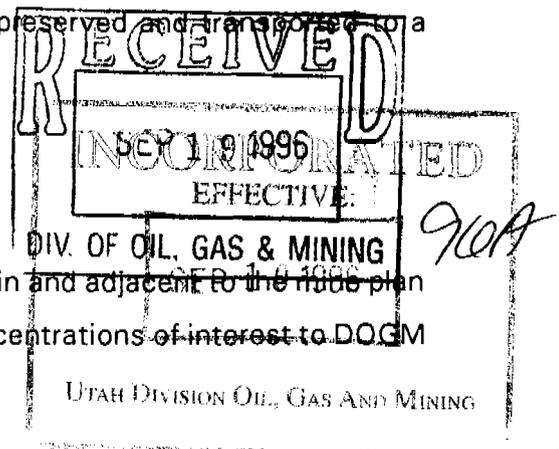
Analyses for dissolved oxygen were discontinued early in 1980. This was done because the concentrations of dissolved oxygen had remained stable since the summer of 1979. Table 7-15 lists the parameters that were monitored from January 1980 through March 1984.

The USGS has maintained stream flow gaging stations (from which daily flow records are available) on all perennial streams located within the mine plan area. The locations of these gaging stations are illustrated on Exhibit 7.2. Two gaging stations have been maintained on Willow Creek, one upstream from the mine plan area (Willow Creek near Castle Gate) and one near the confluence of Willow Creek with the Price River (Willow Creek at Castle Gate). The gaging station on the Price River (Price River near Heiner) is located near the downstream limit of the mine plan area.

**Data Collection** - All water quality data was collected, field tested, and handled for shipment by qualified personnel. Stream flow is determined in the field at the time of sample collection by determining average velocity, width and depth of channel, and applying the appropriate factor for the type of stream channel and flow (a factor of 0.85 is applied to stream flow data from the Price River drainage area). Air temperature, water temperature, and pH were also determined in the field by acceptable methodology. Approximately 5 liters of water was collected at each sampling location, properly preserved and transported to a certified laboratory for the remaining analyses.

### 7.3-1 Groundwater Quality

A summary of groundwater quality data collected within and adjacent to the mine plan area with maximum, minimum, and average of parameter concentrations of interest to DOGM



are presented Appendix 7-2. Also included are the raw groundwater quality data in chronological order (Appendix 7-4).

The data collected so far provides a representative description of groundwater baseline conditions in the mine plan area. General comments are presented below regarding parameters which were high or variable over time:

**Conductivity** - Values of conductivity varied greatly over the mine plan area, ranging from a high of 5150  $\mu\text{mhos/cm}$  at Station B-001 to a low of 390  $\mu\text{mhos/cm}$  at Station B-33. Variability is felt to be tied to the variability of the natural formations.

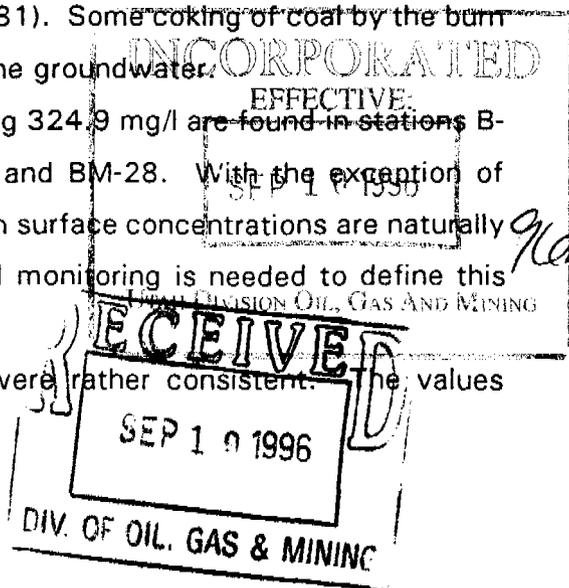
**Total Dissolved Solids** - TDS concentrations varied in a similar fashion to conductivity, mainly for the same reasons. Concentrations ranged from a high of 4420 mg/l to a low of 255 mg/l for stations B-001 and B-33, respectively.

**Oil and Grease** - Concentrations of oil and grease were found in varying concentrations ranging from 25,000 mg/l in well B-43 to 1.0 mg/l at stations B-001 and B-33. The high concentration in well B-43 may indicate that the well has not been developed and prepared correctly to be used as hydrologic monitoring well. Oil and grease concentrations were also found in undisturbed areas indicating the presence of concentrations in the natural environment.

**Phenol** - Moderately high concentrations of phenols averaging 0.025 mg/l are found at stations B-32, B-42, and B-43. Station B-32 is located in Mathis Canyon, an undisturbed area, and indicates naturally occurring phenols. Stations B-42 and 43 are wells that monitor water at depth in the Blackhawk Formation. The phenols in the natural environment are probably a constituent of the natural residue of coal coking (Sawyer & McCarthy, 1964). The coal seam outcrop in the mine plan area shows evidence of coal burns. These burns range 500 to 1000 feet back from the outcrop (Stephens, 1981). Some coking of coal by the burn is very likely, and the residue would be picked up by the groundwater.

**Sulfate** - High concentrations of sulfate averaging 324.9 mg/l are found in stations B-22, B-33, B-41, B-42, B-43, BM-25, BM-26, BM-27, and BM-28. With the exception of springs B-22 and B-33, the potential does exist that high surface concentrations are naturally occurring and associated with coal seams. Additional monitoring is needed to define this occurrences.

All other parameters which were monitored were rather consistent. The values measured for these parameters cause no concern.



### 7.3-2 Surface Water Quality

A summary of surface water quality data containing the maximum, minimum, and average of parameters of interest to DOGM are presented in Appendix 7-5. The raw surface water quality data are presented in Appendix 7-6.

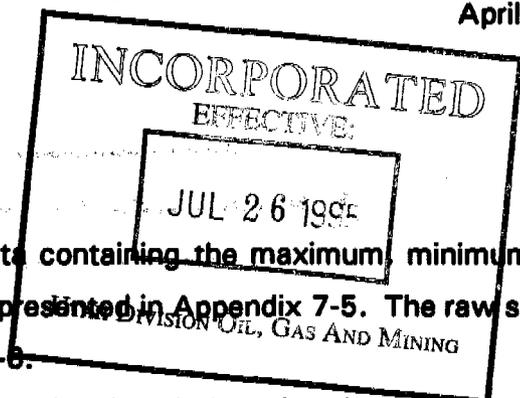
The data collected to date give a representative description of surface water baseline conditions in the mine plan area. The following general comments are given regarding some of the measured parameters.

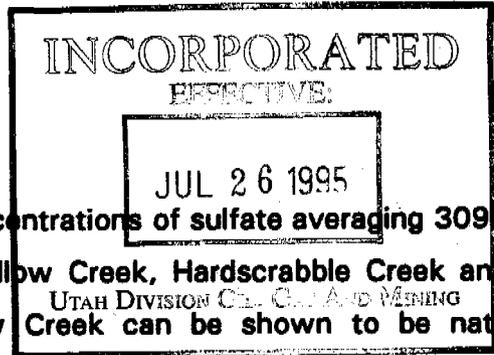
**pH** - Measurements of pH are variable, ranging from a high of 9.6 to a low of 6.9 across the mine plan area. High pH values are seen above and below the Castle Gate Preparation Plant on the Price River. Hardscrabble Creek also had some high values. The variability of the results and the presence of high pH above and below disturbed areas indicate that mining activities are not the cause of high pH measurements.

**Dissolved Oxygen** - Low levels of dissolved oxygen averaging 4.1 mg/l were found in Hardscrabble Creek early in the monitoring program. Since the summer of 1979, there has been a decrease in the impact on the creek. Dissolved oxygen levels have increased to acceptable levels averaging 6.1 mg/l. This is felt to be caused by implementation of sediment control measures in the canyon.

**Oil and Grease** - During the early part of the monitoring program, high oil and grease concentrations averaging 15.3 mg/l were found in Hardscrabble Creek due to mining activities. Similar to dissolved oxygen concentrations mentioned above, oil and grease concentrations have been markedly reduced since the summer of 1979. Oil and grease concentrations averaged 10.2 mg/l since the summer of 1979. Implementation of sediment control measures is felt to be the cause of the reduction.

**Phenol** - Moderately high concentrations of phenol averaging 0.02 mg/l are found in Hardscrabble Creek at the confluence of Sulfur Canyon Creek, and the Price River and in the Price River itself. These concentrations probably result from natural coking of coal and consequently are naturally occurring.





**Sulfate** - High concentrations of sulfate averaging 309.7 mg/l have been found to be occurring regularly in Willow Creek, Hardscrabble Creek and Sowbelly Gulch. The high concentrations in Willow Creek can be shown to be naturally occurring due to high concentrations above the disturbed area. Concentrations in Hardscrabble Creek and Sowbelly Gulch are probably originating from waters in the coal seam.

**Total Dissolved Solids** - Total dissolved solid concentrations averaging 778.8 mg/l have been found in Willow Creek, Hardscrabble Creek, and Sowbelly Gulch. The high TDS concentrations in Willow Creek can be shown to be naturally occurring due to high concentrations above the disturbed area. Concentrations in Hardscrabble Creek and Sowbelly Gulch probably originate from mine water discharge at the mine facilities.

**Fluoride** - Moderate fluoride concentrations averaging 1.9 mg/l have been found regularly in the holding pond below the No. 2 mine. Occasionally concentration levels ranging up to 1.6 mg/l have been observed in Hardscrabble Creek below the No. 3 mine. The fluoride concentrations are probably naturally occurring.

**Iron** - High total iron concentrations averaging 18.25 mg/l have been found in Willow Creek, Hardscrabble Creek, in the Price River above Sulfur Canyon Creek. These concentrations are all felt to be naturally occurring due to the association of iron with suspended solids in the river flows. The cementing agents of the Blackhawk Formation have high iron concentrations (Vaughn Hansen Associates, 1979).

All other parameters which were monitored have been either consistently below state standards for Class "C" waters, or parameters were rather consistent. The values measured for these parameters cause no concern.

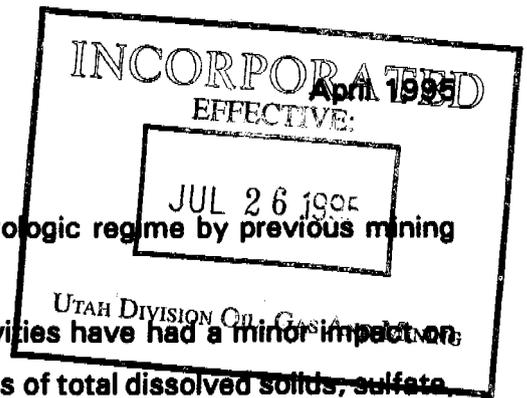
### **7.3-3 Impact of Mining Activities**

The Price River (now Castle Gate Coal Company) mine plan area has been actively mined for the last 85 years (Doelling, 1972). Extensive areas have been undermined by mining in the various coal seams in the area. Determination of the impact of the Price River

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mining activities versus the cumulative impact to the hydrologic regime by previous mining is difficult to separate.

As indicated previously, the surface operation activities have had a minor impact on the hydrologic balance of the mine plan area. Concentrations of total dissolved solids, sulfate and oil and grease have been found in areas associated with mining in fairly high concentrations. However, monitoring over the last one and one-half years has indicated that these concentrations are being reduced as the mine facilities are brought into compliance. The initiation of sediment control measures on the mine sites has markedly reduced impacts on the hydrologic balance.



**7.4 WATER RIGHTS AND REPLACEMENT**

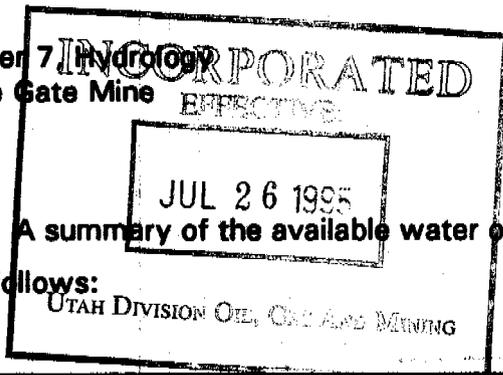
No known private water supplies exist within the coal bearing portions of the Mine Plan Area, and, thus, dewatering of individual supplies is unlikely.

However, in the event of loss of either surface or sub-surface water rights by any party as a result of Castle Gate Coal Company mining activity, Castle Gate will replace and provide (through assignment or transfer of its existing water rights to the party), water of quality and quantity comparable to that which was lost.

**7.4-1 Alternative Water Supply Information**

There are four basic sources of water available to meet the mining requirements as projected. These sources are as follows:

1. Direct Floor Rights (Price River)
2. Reservoir Rights (Schofield Reservoir)
3. Mining, Springs and Other Sources
4. Price River Water Improvement District

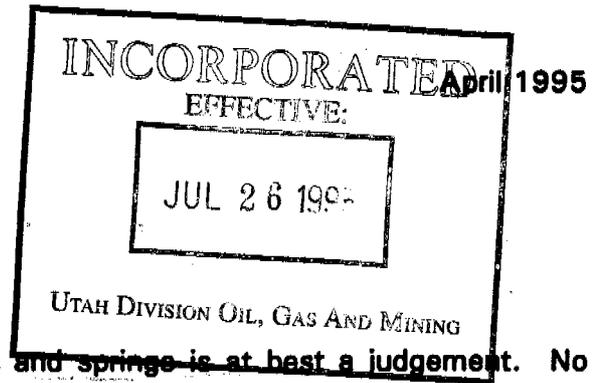


A summary of the available water owned or optioned by Franklin from these sources is as follows:

Direct Flow Rights (Source)	Class	Claim	Flow (cfs)	Comments
Price River (Owned)	1st	3013	0.994	Morse Decree: 1874 priority one of earliest rights on Price River, one of two rights not limited to irrigation season. Use: Industrial, mining and domestic
	2nd	3584	0.20	Morse Decree: 1876 priority
	7th	3585	0.228	2nd and 7th Class rights can only be used if flow of water in Price River is sufficient to satisfy 1st Class rights enumerated in Morse Decree.

Reservoir Rights	Schofield Shares	Comments
Spring Glen Canal Co. (Owned)	39	Acquired from Kanawha & Hocking Coal & Coke Co. on Dec. 28, 1973. Represented by 336.47 shares of Spring Glen Canal Co. Use is dependent on filing and receiving approval for change of use and point of diversion with the Canal Company.
Optioned: 1. Price River Water Users Association (PRWUA)	300	Acquired from Kanawha & Hocking Coal & Coke Co.
2. PRWUA	250	MCO acquired from Marshing on Feb. 11, 1975. Purchase contingent on seller filing and received approved change application with State Engineer to allow industrial use for irrigation water along with change in point of diversion.

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 Castle Gate Mine



**7.4-2 Mine, Springs and Other Sources**

The availability of water from our mines and ~~springs is at best a judgement~~. No definitive studies have been completed recently to measure either the sustained flow rates at our mines or springs, or the quantities of water stored within our mines. It must be noted that the flow rates in the field claims are not absolute and must not be relied upon as a measure of the available water.

The following summary of filed water users' claims, along with abstracts of letters written in prior years, represent the current knowledge about additional potential sources of water for the mining operations.

Additional water rights evidenced by water users' claims owned by Franklin are as follows:

Source (Castle Gate Project)	Claim(s)	Flow (cfs)	Comments
Kenilworth Mine Tunnel	2592	0.923	1882 priority, type of right: Underground Water (UGW). Use: Industrial
Castle Gate Mine Tunnel #2	3593	1.810	1892 priority, Right: 1892 priority, Right: UGW. Use: Industrial
Castle Gate Mine Tunnel #3	3594	0.545	1925 priority, Right: UGW. Use: Industrial
Royal Mine Tunnel	324	0.668	1936 priority, Right: UGS Use: Industrial
Total Castle Gate Project		3.946 cfs	

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Source (Spring Canyon Project)	Claim(s)	Flow (cfs)	Comments
Spring Canyon Creek	26, 29 & 62	0.230	See Note 1
Burnt Tree Fork	25, 27, 34 & 40	0.155	See Note 1
Mine Tunnel	325	1.114	Right: UGW (See Note 2)
Gravel Spring	18	0.455	See Note 2
Unnamed Spring	30	0.004	See Note 2
<b>Total Spring Canyon Project</b>		<b>1.958 cfs</b>	<b>All used for industrial and domestic.</b>

Source (Carbon Fuel Project)	Claim(s)	Flow (cfs)	Comments
Carbon Fuel	3858	N/A	Right: UGW (See Note 5)

**NOTES:**

1. These water user claims are subject to conditions of court decree signed by Judge Christensen, December 19, 1914.
2. Price River Coal Company will have exclusive use of all water at Gravel Spring (Claim #18) and Mine Tunnel (Claim #325) for industrial and domestic purposes per Christensen decree. As a result of recent efforts (late spring, 1975) to redevelop the flow from this spring for use in the No. 5 mine, a maximum sustained flow rate of 50 gallons per mine (0.111 cfs) has been realized during the month of May, 1975.
3. Legal filings were made during the fall of 1974 with the State Water Engineer to re-establish the validity of these claims.
4. The value of any of these water rights is dependent primarily upon the ability of these sources to deliver water at the rates claimed on a sustained basis. If the supply is firm and can be depended upon, these rights become very valuable. It must be emphasized that no measurements on a sustained basis (other than Gravel Spring during May, 1975) have been performed recently; therefore, the claimed rates of flow per the water users claims are, at best, arbitrary.

5. Memo of February 4, 1936, by L.P. Pearce (Engineer, Independent Coal & Coke). Subject: Possibility of developing water supply at Kenilworth. Summary: No groundwater: possibility of 20,000 gallons per day (gpd) surplus mine water, not effective springs.
6. Letter of December 5, 1956, by L.L. Arnett (Coal Industrial Board of Review, State of Utah). Subject: Proposed plan for water storage and pumping in Castle Gate Mine (set forth by W.L. Potter, November 28, 1956). Summary: Granted, provided barrier pillar 75 feet thick maintained along with concrete dams being built.
7. Letter of August 7, 1968, by W. Guy (North American Coal Company). Subject: Water contained in inactive mines.

Summary

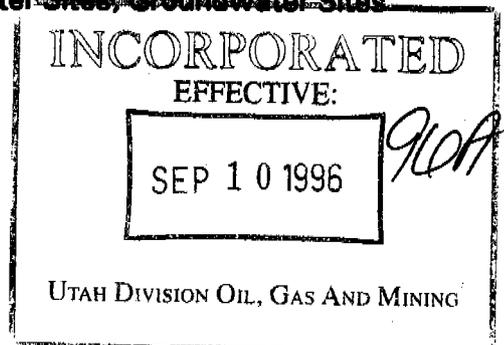
Castle Gate No. 2 Mine (1,313 acre feet)	428 million gallons
Castle Gate No. 3 Mine (392 acre feet)	128 million gallons
Kenilworth A Seam (506 acre feet)*	165 million gallons

- \* Per measurements made from Kenilworth Seam (active estimate 100 gpm (0.22 cfs) steady flow.

**7.5 OPERATIONAL WATER QUALITY MONITORING PLAN - APRIL 25, 1984**

The following operational water quality monitoring program outline is a coalescence of Castle Gate Coal Company's water monitoring program since 1978, proposals made during 1983-1984 by Price River Coal Company, and various discussions and requirements with and by the Office of Surface Mining during 1984.

Three kinds of monitoring will be conducted: ~~Surface Water Sites, Groundwater Sites~~ and Active Mine Monitoring.



**7.5-1 Surface Water Monitoring**

Surface water stations will be sampled four (4) times per year on or about the following dates (plus or minus 2 weeks):

March 15th  
June 10th  
Sept. 5th  
Nov. 30th

These dates are chosen to attempt to coincide, respectively, with first thaw, Spring high flow, end of Summer low-flow and a last sample before freeze up. Samples are generally unobtainable from early December to late February due to freeze up and inaccessible roads.

The eight (8) sample points will be those indicated on Exhibit 7-3 (B-5 (Price River), B-6 (Price River), B-11 (Hardscrabble Canyon), B-12 (Hardscrabble Canyon), B-25 (Crandall Canyon), B-26 (Crandall Canyon), B-45 (Sowbelly Canyon) and B-28 (Sowbelly Canyon). In August 1996, sample point B-17 was dropped and a replacement point, B-45 was added up-canyon at the gate in Sowbelly Gulch.

Points B-3 and B-27 were dropped from the Permit when Castle Gate Coal Company acquired the Western Reserve in May, 1986.

Sampling will combine field analyses of certain parameters and the obtaining of a bottled sample for abbreviated laboratory analysis. Parameters to be included in the active monitoring program are listed in Table 7-16.

Sampling and analysis will be done according to the current edition of "Standard Methods for the Examination of Water and Wastewater". Sample reports will be provided to DOGM for each sampling period. A summation will be provided at the end of each sampling year.

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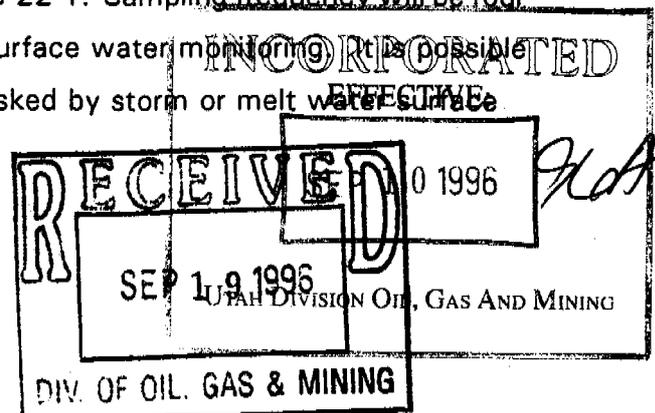
Surface water sampling for Hardscrabble, Sowbelly, Crandall and Castle Gate will continue until bond is released, as required by R645-301-731.224. Two years after the area has been seeded, the water quality will be evaluated to determine: 1) if the surface water leaving the reclaimed area is suitable to support the postmining land uses, and 2) if monitoring is no longer necessary to achieve the purposes set forth in the monitoring plan consistent with R645-301-731.224.1 and R645-301-731.224.2.

In order to establish that the runoff from the reclaimed areas will not degrade the quality of the receiving stream waters outside the permit area (R645-301-731.121), additional monitoring will be done for a six month period prior to the request to abandon a sediment pond. Samples will be taken at the inlets to ponds 011, 012, 013, 014, and 015. The samples will be tested for field parameters and abbreviated laboratory analysis as described in this section.

Because of the small drainage areas flowing into the sediment ponds, there is very little or no flow into the ponds except during very large storm events. In the event that there is not enough flow into a pond to get an adequate sample, then a sample of the water in the pond will be used to perform the analysis.

### 7.5-2 Groundwater Sampling

Groundwater sampling will be carried out at the four (4) points indicated on Exhibit 7-3; B-22-1 (Crandall Canyon), and BM-30 (Adit No. 1), BM-31 (Near Mutual Spring), and B-32 (Gravel Springs). Point BM-29 was dropped by Castle Gate Coal Company after acquisition of the property. Sample point B-22 was dropped in the summer of 1996, following some discrepancies over its location and was replaced with B-22-1. Sampling frequency will be four (4) times per year and on dates similar to those for surface water monitoring. It is possible that flows from points B-22-1 and B-32 could be masked by storm or melt water surface



flows, distorting the true values and disrupting long term evaluations. Should this occur, sampling will be withheld until the event passes. Sample analyses are listed in Table 7-16. Quarterly reports and annual summations will be provided. Castle Gate will follow sample procedures as outlined in Division policy for surface and groundwater sampling.

Groundwater will be sampled for a period of two years after sealing of the mine unless significant changes in quality of the water occurs from previous years of data.

### 7.5-3 Active Mine Monitoring

It has been observed and generally agreed upon that Castle Gate No. 5 and No. 3 are usually dry. Some monitoring, however, may be appropriate to satisfy the regulatory requirements. The intent of in-mine monitoring is to identify and analyze significant groundwater flows, should they be encountered, so as to try to assess the impact of mining upon the quantity and quality of the groundwater resource.

Most in-mine water is generated at the working face. Sampling these areas is not only unsafe but also unrepresentative of groundwater flows or quality due to the mining machine dust sprays which can deliver 80 to 200 gpm. "Leakers" appearing at the face usually disappear as the mining proceeds, within a few hours or days. Small flows (3-5 gpm) are often immeasurable. They may seep down a rib and be absorbed by rock dust or rapidly evaporated away in the ventilation system. A discernible flow may include combined seepage points from several or even several dozen mining sections.

Water from the mine is collectively pumped to the surface and discharged into the process water pond after the water passes the water treatment system. The treatment system is located in the preparation plant area, Exhibit 3.4-1. The water treatment system is monitored and maintained daily by Culligan of Price (when the mine is in operation and the treatment system is necessary based on the quality of water). The plant requires chemicals to be mixed and injected continually into four large regenerant tanks. These softeners remove the oil present in the mine water, as well as some of the total dissolved solids.

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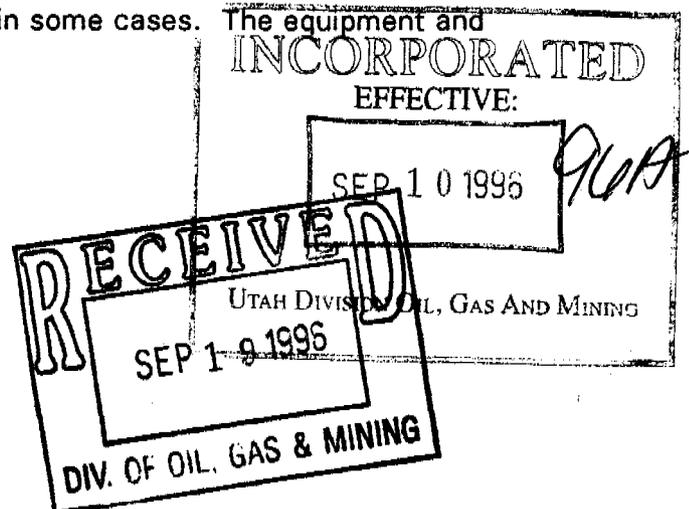
Flocculent, a material that bonds oil particles together, is injected into the system so that the mixed bed softeners can remove the oil from the water as it passes through. These are control timers on each of the four units that regulate the regeneration time and frequency. The clean water then reports to Pond 012.

A totalizing flow meter will be installed at the point of discharge into the process water pond, therefore the quantity of water discharged from the mine will be recorded. An annual monitoring report of the mine discharge water will be prepared.

Field parameters will be measured quarterly. Every quarter bottled samples will be collected and analyzed as per the abbreviated lab analysis schedule. The parameters analyzed for are listed in Table 7-16. The locations of the in-mine water sampling locations are shown on Exhibit 7-5. Trace metals will be analyzed every two (2) years. The reports will be summarized on a yearly basis. They will be kept at the mine and available for inspection upon request.

#### 7.5-4 Water Budget Analysis

A mine water budget analysis will be calculated along with the annual monitoring report summation. The budget analyses will be an attempt to assess the quantities of water removed or discharged from the mines and the quantities pumped into the mines or generated from groundwater storage. Estimates made will be very general "order of magnitude" calculation and not based on exact measurement in some cases.



personnel needed for precise measurement of all factors relating to water movement are prohibitively expensive for the type of analyses needed.

Several factors affect the movement of water into and out of the mines. These are:

INFLOW SOURCES

- 1. Pumped in make-up water
- 2. Groundwater inflow
- 3. Water vapor through ventilation system

OUTFLOW SOURCES

- 1. Adherent to coal
- 2. Water vapor through ventilation system
- 3. Pumped out

As a matter of interest, we have made a rough calculation on the water budget for the inactive No. 3 Mine. From various underground measurements, average temperature is 58 degrees (F); average mean temperature outside is 41 degrees (F), (taken from monthly average temperatures for gas consumption - Crandall Canyon facilities by Horrocks Engineers); mean relative humidity of intake air is 54% (personal communication with U.S. National Weather Service Forecast, Salt Lake City); measurement at the fan reveals air replacement to be 450,000 cfm through the mine. From the handbook "Mine Ventilation", Mining Engineering 505, University of Utah by J. Wilson, grains of moisture per cubic foot of air at 58 degrees F is 5.424, and at 41 degrees F it is 2.970. Using the above parameters, the amount of water taken out of the mine by the ventilation system, assuming 95% relative humidity at the fan is:

$$\frac{(5.24 \times 0.95 - 2.970 \times 0.54) \times 450,000 \times 60 \times 24 \times 0.1198}{7,000} = 39,359 \text{ gpd}$$

Since No. 3 mine was making about 34 gpm (1-83) = 30 x 60 x 24 = 48,960 gpd, it is obvious that with no moisture leaving by adherence to coal, the mine has a net accretion of 96000 gpd and must be pumped to avoid flooding the down dip sections.

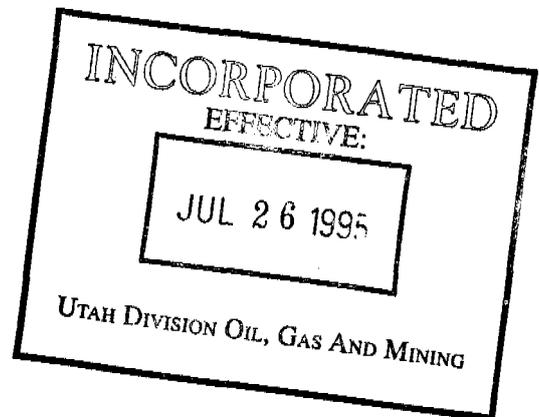
Various samples taken from the raw coal feed from the mines indicate 10% free moisture adhering to the R.O.M. coal. This percentage to be used in the future water budget calculations converts to a 24 gallon removal per ton of coal mined. For example, should the

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No. 3 mine reactivate with a longwall and two development sections producing 3,000 tons per day (tpd), the water removal would be in the range of 72,000 gpd.

Water pumped into the mines is directly related to tons of coal mined, since ventilation losses are relatively constant.

The last monitoring event under the baseline program took place on March 7, 1984. Price River Coal Company began the operational monitoring in June, 1984. Castle Gate Coal Company will continue the monitoring.



**7.6 REFERENCES**

Adair, Laine. **Price River Coal Company**, Helper, Utah, March 21, 1983. Personal communication

American Iron and Steel Institute. 1983. Handbook of Steel Drainage & Highway Construction Products. American Iron and Steel Institute. Washington, D.C.

Barfield, B.J., R.C. Warner, and C.T. Haan. 1981. Applied Hydrology and Sedimentology for Disturbed Areas. Oklahoma Technical Press. Stillwater, Oklahoma.

Bathurst, J.C., R.M. Li, D.B. Simons, 1979, Hydraulics of Mountain Rivers. Civil Engineering Department, Colorado State University, CER78-79JCB-RML-DBS55.

Chow, V.T. 1959. Open Channel Hydraulics. McGraw-Hill Book Company. New York City, New York.

Clark, Frank R. 1928. Economic Geology of Castlegate, Wellington and Sunnyside Quadrangles, Carbon County, Utah. U.S. Geological Survey Bulletin 793.

Cordova, R.W. 1964. Hydrogeologic Reconnaissance of Part of the Headwaters Area of the Price River, Utah Geological and Mineralogical Survey Water Resources Bulletin No. 4. Salt Lake City, Utah.

Danielson, T.W., M.D. ReMillard, and R.H. Fuller. 1981. Hydrology of the Coal-Resource Areas in the Upper Drainages of Huntington and Cottonwood Creeks, Central Utah, U.S. Geological Survey Open File Report 81-539. Salt Lake City, Utah.

Doelling, H.H. 1972. Wasatch Plateau Coal Fields, In Doelling, H.H. (ed.), Central Utah Coal Fields; Sevier-Sanpete, Wasatch Plateau, Cook Cliffs and Emery, Utah Geological and Mineralogical Survey Monograph Series No. 3. Salt Lake City, Utah.

Elegante, Berths. Principal water right holder on Crystal and Goat Springs, May 12, 1983. Personal communication.

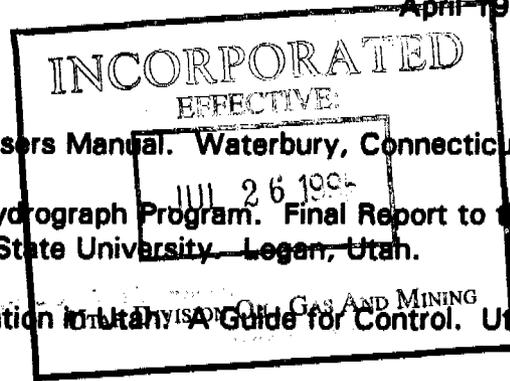
Freeze, R. Allan, and John A. Cherry. 1979. Groundwater. Prentice-Hall, Inc. Englewood Cliffs, New Jersey.

Geocomp Corporation. 1988. GEOSLOPE, Version 4.2. Concord, Mass.

Golder Associates, Bellevue, Washington. January 20, 1983. Correspondence from Charles W. Lockhart (Golder Associates) to Robert Wiley, Price River Coal Company.

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April 1995



- Haestad Methods, Inc. 1990. FLOWMASTER I - Users Manual. Waterbury, Connecticut.
- Hawkins, R.H. and K.A. Marshall. 1979. Storm Hydrograph Program. Final Report to the Utah Division of Oil, Gas and Mining. Utah State University. Logan, Utah.
- Israelsen, C.E., et al. 1984. Erosion and Sedimentation in Utah. A Guide for Control. Utah Water Research Laboratory. Logan, Utah.
- Jeppson, R.W., et al. 1968. Hydrologic Atlas of Utah. Utah Water Research Laboratory and State of Utah Department of Natural Resources, PRWG635-1. Utah State University. Logan, Utah.
- Miller, J.F., R.H. Frederick, and R.J. Tracey. 1973. Precipitation-Frequency Atlas of the Western United States. Volume VI-Utah. National Oceanic and Atmospheric Administration. National Weather Service. Silver Spring, Maryland.
- Price, D. and T. Arrow. 1974. Summary Appraisals of the Nation's Ground Water Resources -Upper Colorado Region. U.S. Geological Survey Professional Paper 813-C. Washington, D.C.
- Price, E. and K.M. Waddel. 1973. Selected Hydrologic Data in the Upper Colorado River Basin. U.S. Geological Survey Hydrologic Investigations Atlas HA-477. Washington, D.C.
- Sawyer, C.N. and P.L. McCarthy. 1964. Chemistry for Sanitary Engineers, 2nd Edition. McGraw-Hill Book Company. p 518.
- Searcy, J.K. 1967. Use of Riprap for Bank Protection. Hydraulic Engineering Circular No. 11. U.S. Department of Transportation. Washington, D.C..
- Simons, Li and Associates, Inc. September 1982. OSM/TR-82/2. Surface Mining Water Diversion Design Manual. Prepared for the Office of Surface Mining. Washington, D.C..
- Spieker, E.M. 1925. Geology of Coal Fields in Utah. U.S. Bureau of Mines Technical Paper 345, pp. 13-72.
- Stephens, D. 1981. Price River Coal Company, Geologist. Personal communication with Thomas Suchoski of Vaughn Hansen Associates.
- Suchoski, T.J. 1990. Computer Program for Circular Open Channel Flow. HYDRO/CALC. Douglas, Wyoming.

INCORPORATED  
EFFECTIVE:

U.S. Department of Transportation. 1977. Hydraulic Charts for the Selection of Highway Culverts. Hydraulic Engineering Circular No. 5. Federal Highway Administration. Washington, D.C.

U.S. Department of Transportation. 1978. Use of Riprap for Bank Protection. Hydraulic Engineering Circular No. 11. Federal Highway Administration. Washington, D.C.

U.S. Geological Survey. 1976. Environmental Impact Analysis of the Braztah Corporation Proposal to Mine up to Six Million Tons per year of Coal near Helper, Utah. October 21, 1976, Memorandum from the District Chief, WRD, Salt Lake City, Utah to the Area Mining Supervisor, Conservation Division, Salt Lake City, Utah.

U.S. Geological Survey. 1979. Development of Coal Resources in Central Utah - Final Environmental Statement. Salt Lake City, Utah.

U.S. Soil Conservation Service. 1968. Hydraulics of Broad-Crested Spillways. Technical Release No. 39. U.S. Government Printing Office. Washington, D.C.

U.S. Soil Conservation Service. 1972. National Engineering Handbook, Section 4: Hydrology. U.S. Government Printing Office. Washington, D.C.

Utah Department of Highways Manual of Instruction, Part 4: Roadway Drainage. 1965.

Utah Division of Oil, Gas and Mining. 1987. Utah Coal Mining and Reclamation Regulatory Program. Rules Pertaining to Underground Coal Mining Activities, Chapter I. Utah Department of Natural Resources. Salt Lake City, Utah.

Utah Division of Oil, Gas and Mining. August 23, 1991. State of Utah R645 - Coal Mining Rules. Department of Natural Resources. Salt Lake City, Utah.

Vaughn Hansen Associates. 1979. Water Monitoring Program.

Warner, R.C. and Schwab, P.J. 1992. SEDCAD+ Version 3 computer software. University of Kentucky and Civil Software Design.

Warner, R.C., et al. 1980. A Hydrology and Sedimentology Watershed Model, Part II: Users' Manual. (SEDIMONT II). Department of Agricultural Engineering. University of Kentucky. Lexington, Kentucky.

Weider, M.F., K.G. Kirk, and L.E. Welborn. 1983. Simplified Analysis Routines for Surface and Groundwater Hydrology Applications in Surface Mining. Proceedings of the 1983 Symposium on Surface Mining Hydrology, Sedimentology, and Reclamation. University of Kentucky. Lexington, Kentucky.

Wilson, B.N., B.J. Barfield, and I.D. Moore. 1980. A Hydrology and Sedimentology Watershed Model, Part I: Modeling Techniques. Department of Agricultural Engineering. University of Kentucky. Lexington, Kentucky.

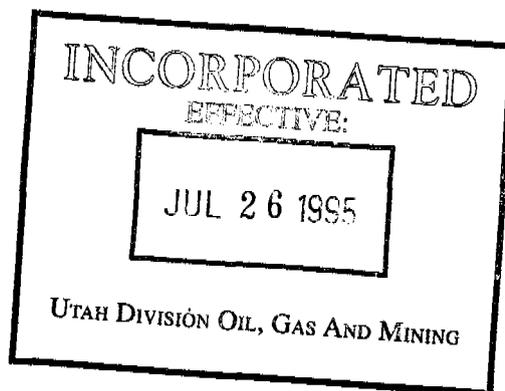


Table 7-1. Aquifer test results in the Blackhawk Formations as derived from Homes MC-205, MC-206, and MC-207.

Drill Hole No.	Thickness Zone Tested Feet	Average Hydraulic Conductivity ft/sec.	Total Transmissivity gpd/ft	Range of Hydraulic Conductivity ft/sec.-ft/sec.
MC-205	651	$4.7 \times 10^{-6}$	65	$2.03 \times 10^{-8}$ - $3.02 \times 10^{-7}$
MC-206	808	$3.2 \times 10^{-8}$	17	$3.94 \times 10^{-9}$ - $3.94 \times 10^{-8}$
MC-207	-	-	-	$7.55 \times 10^{-8}$ - $1.15 \times 10^{-7}$

Table 7-2. Seepage rates into the No. 3 Mine (Sub 3 Seam) and the No. 5 Mine (Castlegate D Seam), data obtained from Laine Adair, 1983.

<u>Measurement Site</u>	<u>Contributing Area Within Mine acres</u>	<u>Flow Rate 11/3/82 gpm</u>	<u>Discharge per Unit Area 11/3/82 gpm/acre</u>	<u>Flow Rate 1/26/83 gpm</u>	<u>Discharge Per Unit Area 1/26/83 gpm/acre</u>
<b>No. 3 Mine (Sub 3 Coal Seam)</b>					
1st Raise	25	4.5	0.18	3.5	0.14
3rd West	26	9.1	0.35	4.5	0.17
4th West	11	8.2	0.75	3.1	0.28
MNW	196	3.0	0.03	0.6	0.01
MNE		3.8		1.4	
Main Dips	80	0.5	0.01	0.5	0.006
9th East	16	12.0	0.13	2.2	0.14
7th & 8th East	396	13.4	0.03	13.4	0.037
Main Slope & 2nd East	170	4.2	0.02	4.2	0.035
<b>TOTAL</b>	<b>920</b>	<b>48.7</b>	<b>0.05 Ave.</b>	<b>33.4</b>	<b>0.04 Ave.</b>
<b>No. 5 Mine (Castlegate D Seam)</b>					
4th West Seal	74	-	-	1.5	0.020
10th West	158	-	-	2.0	0.013
<b>TOTAL</b>	<b>232</b>			<b>3.5</b>	<b>0.015 Ave</b>

Table 7-3. Ground water recharge to accessible abandoned and existing coal mines within the mine plan area.

A - Recharge to Abandoned Mines

<u>Mine</u>	<u>Measured Inflow gpm</u>	<u>Mine Area acres</u>	<u>Yield/Unit Area gpm/acre</u>	<u>Recharge in./yr</u>
Aberdeen	2.9	770	0.004	0.08
Utah Fuel No. 1	12	680	0.018	0.35
Royal	30	1250	0.024	0.46
Kennilworth	30	2790	0.011	0.21

B - Recharge to Castle Gate Coal Company's Carbon County Mines

<u>Mine</u>	<u>Measured Inflow gpm</u>	<u>Mine Area acres</u>	<u>Yield/Unit Area gpm/acre</u>	<u>Recharge in./yr</u>
No. 3 (11/3/82)	48.7	920	0.053	1.02
No. 3 (1/26/83)	33.4	920	0.036	0.70
No. 5 (1/26/83)	3.5	232	0.015	0.29

TABLE 7-4

NUMERICAL STANDARDS FOR PROTECTION OF BENEFICIAL USES OF WATER

Constituent	CLASSES											
	Domestic Source			Recreation & Aesthetics		Aquatic Wildlife			Agri- culture	Indus- try	Special	
	1A	1B	1C	2A	2B	3A	3B	3C	3D	4	5	6
<b>Microbiological (No./100 ml)</b>												
(30-day Geometric Mean)												
Maximum Total Coliforms	1	50	5,000	1,000	5,000	•	•	•	•	•	•	•
Maximum Fecal Coliforms	•	•	2,000	200	2,000	•	•	•	•	•	•	•
<b>Physical</b>												
Total Dissolved Gases	•	•	•	•	•	(b)	(b)	•	•	•	•	•
Minimum DO (mg/l) (a)	•	•	5.5	5.5	5.5	6.0	5.5	•	5.5	•	•	•
Maximum Temperature	•	•	•	•	•	20°C	27°C	•	•	•	•	•
Maximum Temp. Change per	•	•	•	•	•	2°C	4°C	•	•	•	•	•
Turbidity Increase (c)	6.5-9.0	6.5-9.0	6.5-9.0	6.5-9.0	6.5-9.0	6.5-9.0	6.5-9.0	•	6.5-9.0	6.5-9.0	•	•
	•	•	•	10 NTU	10 NTU	10 NTU	10 NTU	•	15 NTU	•	•	•
<b>Chemical (Maximum mg/l)</b>												
Arsenic, dissolved	.05	.05	.05	•	•	•	•	•	•	•	.1	•
Barium, dissolved	1	1	1	•	•	•	•	•	•	•	•	•
Calcium, dissolved	.010	.010	.010	•	•	.0004(d)	.004(d)	•	•	•	.01	•
Chromium, dissolved	.05	.05	.05	•	•	.10	.10	•	.10	•	.10	•
Copper, dissolved	•	•	•	•	•	.01	.01	•	•	•	.2	•
Cyanide	•	•	•	•	•	.005	.005	•	•	•	•	•
Iron, dissolved	•	•	•	•	•	1.0	1.0	•	1.0	•	•	•
Lead, dissolved	.05	.05	.05	•	•	.05	.05	•	•	•	.1	•
Mercury, total	.002	.002	.002	•	•	.00005	.00005	•	.00005	•	•	•
Phenol	•	•	•	•	•	.01	.01	•	•	•	•	•
Selenium, dissolved	.01	.01	.01	•	•	.05	.05	•	•	•	.05	•
Silver, dissolved	.05	.05	.05	•	•	.01	.01	•	•	•	•	•
Zinc, dissolved	•	•	•	•	•	.05	.05	•	•	•	•	•
NH <sub>3</sub> as N (un-ionized)	•	•	•	•	•	.02	.02	•	•	•	•	•
Chlorine	•	•	•	•	•	.002	.01	•	•	•	•	•
Fluoride, dissolved (e)	1.4-2.4	1.4-2.4	1.4-2.4	•	•	.002	•	•	•	•	•	•
as N	10	10	10	•	•	•	•	•	•	•	•	•
Iron, dissolved	•	•	•	•	•	•	•	•	•	•	.75	•
MnS	•	•	•	•	•	.002	.002	•	•	•	•	•
TDS (f)	•	•	•	•	•	•	•	•	•	•	1200	•
<b>Radiochemical (Maximum pCi/l)</b>												
Gross Alpha	15	15	15	•	•	15(g)	15(g)	•	15(g)	15(g)	•	•
Radium 226, 228 combined	5	5	5	•	•	•	•	•	•	•	•	•
Strontium 90	8	8	8	•	•	•	•	•	•	•	•	•
Tritium	20,000	20,000	20,000	•	•	•	•	•	•	•	•	•
<b>Pesticides (Maximum ug/l)</b>												
Endrin	.2	.2	.2	•	•	.004	.004	•	.004	•	•	•
Lindane	4	4	4	•	•	.01	.01	•	.01	•	•	•
Methoxychlor	100	100	100	•	•	.03	.03	•	.03	•	•	•
Toxaphene	5	5	5	•	•	.005	.005	•	.005	•	•	•
2, 4-D	100	100	100	•	•	•	•	•	•	•	•	•
2, 4, 5-T	10	10	.10	•	•	•	•	•	•	•	•	•
<b>Pollution Indicators (g)</b>												
Gross Beta (pCi/l)	50	50	50	•	•	50	50	•	50	50	•	•
BOD (mg/l)	•	•	5	5	5	5	5	•	5	5	•	•
NO <sub>3</sub> as N (mg/l)	•	•	•	4	4	4	4	•	4	4	•	•
PO <sub>4</sub> as P (mg/l)(h)	•	•	•	.05	.05	.05	.05	•	•	•	•	•

STANDARDS WILL BE DETERMINED ON A CASE-BY-CASE BASIS

STANDARDS WILL BE DETERMINED ON A CASE-BY-CASE BASIS

STANDARDS WILL BE DETERMINED ON A CASE-BY-CASE BASIS

TABLE 7-4 (Continued)

NUMERICAL STANDARDS FOR PROTECTION OF BENEFICIAL USES OF WATER

- Insufficient evidence to warrant the establishment of numerical standards. Limits assigned on case-by-case basis.
- (a) These limits are not applicable to lower water levels in deep impoundments.
- (b) Not to exceed 110% of saturation.
- (c) For Classes 2A, 2B, 3A, and 3B at background levels of 100 NTUs or greater, a 10% increase limit will be used instead of the numeric values listed. For Class 3D at background levels of 150 NTUs or greater, a 25% increase limit will be used instead of the numeric value listed. Short term variances may be considered on a case-by-case basis.
- (d) Limit shall be increased threefold if  $\text{CaCO}_3$  hardness in water exceeds 150 mg/l.

- (e) Maximum concentration varies according to the daily maximum mean air temperature.

Temp. °C	mg/l
12.0 and below	2.4
12.1 to 14.6	2.2
14.7 to 17.6	2.0
17.7 to 21.4	1.8
21.5 to 26.2	1.6
26.3 to 32.5	1.4

- (f) Total dissolved solids (TDS) limit may be adjusted on a case-by-case basis.
- (g) Investigations should be conducted to develop more information where these pollution indicator levels are exceeded.
- (h)  $\text{PO}_4$  as P (mg/l) limit for lakes and reservoirs shall be .025.

TABLE 7-5

NUMERICAL STANDARDS FOR PROTECTION OF CLASS 3C WATER USE

<u>Physical</u>	
Minimum D.O. (mg/l)	5*
Maximum Temperature	27 degrees C**
Maximum Temperature Change	4 degrees C
pH	6.5-9.0
Turbidity Increase (NTU)	15****
<u>Chemical (Maximum mg/l)</u>	
Cadmium, dissolved	0.004
Chromium, dissolved	0.1
Copper, dissolved	0.01
Cyanide	0.005
Iron, dissolved	1.0
Lead, dissolved	0.05
Mercury, total	0.0005
Phenol	0.01
Selenium, dissolved	0.05
Silver, dissolved	0.01
Zinc, dissolved	0.05
Chlorine	0.2
H <sub>2</sub> S	0.02
<u>Radiological (Maximum pCi/l)</u>	
Gross Alpha	15
Gross Beta	30
<u>Pesticides (Maximum mg/l)</u>	
Endrin	0.004
Lindane	0.01
Methoxychlor	0.03
Toxaphene	0.005
<u>Pollution Indicators***</u>	
BOD (mg/l)	5.0
NO <sub>3</sub> as N (mg/l)	4.0

\*Minimum D.O. (mg/l) limitation is 4 in the following segments:  
 San Rafael River and tributaries, from confluence with Green River to confluence with Ferron Creek  
 Malad River and tributaries, from confluence with Bear River to state line

\*\*Maximum temperature limitation is 35 degrees C in the following segments:  
 Virgin River and tributaries from state line to headwaters except as listed in APPENDIX B

\*\*\*Investigations should be conducted to develop more information where these pollution indicator levels are exceeded

\*\*\*\*At background levels of 150 NTU's or greater, a 10% increase limit will be used instead of the numeric values. Short term variances may be considered on a case-by-case basis

TABLE 7-6

TRIBUTARY AREAS OF PRINCIPAL WATERSHEDS WITHIN THE  
 MINE PLAN AREA AS DESIGNATED ON EXHIBIT 7-3

INCORPORATED  
 EFFECTIVE  
 8/2/88

UTAH DIVISION OF  
 MINING

WATERSHED	AREA ACRES
Price River	265,600
Willow Creek	49,540
Spring Canyon Creek	14,130
Sowbelly Gulch	1,980
Hardscrabble Canyon Creek	1,800
Crandall Canyon Creek	2,670
Bear Canyon Creek	1,060
Sulphur Canyon Creek	3,800
Ford Creek	2,450

**TABLE 7-7**  
**PRECIPITATION DATA FOR CASTLE GATE MINE COMPLEX**

<b>WATERSHED AREA</b>	<b>FREQUENCY (YRS)</b>	<b>DURATION (HRS)</b>	<b>PRECIPITATION (IN)</b>
<b>Hardscrabble Canyon</b>	10	6	1.4
	10	24	1.9
	25	6	1.6
	25	24	2.3
	100	6	2.0
<b>Sowbelly Gulch</b>	10	6	1.4
	10	24	1.9
	25	6	1.6
	25	24	2.3
	100	6	2.2
<b>Crandall Canyon</b>	10	6	1.4
	10	24	1.9
	25	6	1.6
	25	24	2.3
	100	6	2.0
	100	24	2.9
<b>Preparation Plant Adit No. 1 Canyon Gravel Canyon</b>	10	6	1.4
	10	24	1.8
	25	6	1.6
	25	24	2.3
	100	6	2.0

**INCORPORATED**  
 EFFECTIVE  
 Reference: Miller et al., 1973  
 JUL 26 1995  
 UTAH DIVISION OIL, GAS AND MINING

TABLE 7-8

AREAS THAT DO NOT REPORT TO A SEDIMENT FOND WITHIN THE  
CASTLE GATE MINE COMPLEX - OPERATION PHASE

**INCORPORATED**  
 EFFECTIVE  
 JUL 26 1995  
 DIVISION OF COAL AND MINING

LOCATION	EXHIBIT	DESCRIPTION OF AREA	SIZE (Acres)	ALTERNATIVE SEDIMENT CONTROL (R645-301-742.240)
Sowbelly Canyon	3.2-2A	Substation and access road	2.8	Gravel
Hardscrabble	3.3-3	Substation	0.5	1" clean rock
Hardscrabble	3.3-3	Powder magazine area	1.2	Vegetation, and straw bales or silt fences
Hardscrabble	3.3-3	Dog Flat	1.8	Vegetation
Hardscrabble	3.3-3	Upper bathhouse	0.3	Vegetation around bathhouse, 1" clean rock on parking lot with berm to prevent flow to undisturbed ditch.
CG Preparation Plant	3.4-2	Unit train loadout	0.5	Vegetation
CG Preparation Plant	3.4-2	Raw Water Pond	3.1	Vegetation, gravel, pond
CG Preparation Plant	3.4-2	Truck scale	1.8	Vegetation, asphalt
CG Preparation Plant	3.4-2	Beltline	0.2	Vegetation
Adit No. 1 Canyon	3.5-1	Area below transfer building	0.5	Straw bales or silt fence
Gravel Canyon	3.6-1	Entire area	5.5	Vegetation
Crandall Canyon	3.7-5C	Topsoil stockpile No. 1 and No. 2	1.0	Vegetation
Crandall Canyon	3.7-5A	Leach Field	0.2	Vegetation
Bear Canyon	3.9-1	Degasification well	0.20	Perimeter berm and vegetation

TABLE 7-9

**AREAS THAT DO NOT REPORT TO A SEDIMENT POND WITHIN THE  
 CASTLE GATE MINE COMPLEX - RECLAMATION PHASE<sup>(a)</sup>**

LOCATION	EXHIBIT	DESCRIPTION OF AREA	SIZE (Acres)	ALTERNATIVE SEDIMENT CONTROL (R645-301-742.240)
Sowbelly Canyon	3.2-5	East of SBRD-1C and south of SBRD-2, South of Pond 017, and Substation area.	12	Mulch and silt fences
Hardscrabble Canyon	3.3-5	Entire reclaimed area	39	Mulch and silt fences
CG Preparation Plant	3.4-3A	Truck scale	1.8	Mulch and silt fences
CG Preparation Plant	3.4-3A	Beltline	0.2	Mulch
CG Preparation Plant	3.4-3A	Reclaimed channel below Schoolhouse Canyon	3.2	Mulch and silt fences
CG Preparation Plant	3.4-3A	Disturbed area within CGRWS-U7	6.1	Mulch and silt fences
CG Preparation Plant	3.4-3A	Unit train loadout	0.5	Mulch and silt fences
CG Preparation Plant	3.4-3A	North end of reclaimed area	1.3	Mulch and silt fences
CG Preparation Plant	3.4-3A	West of Pond 011	0.3	Mulch and silt fences
Adit No. 1	3.5-3	Entire reclaimed area	3	Mulch and silt fences
Gravel Canyon	3.6-3	Entire reclaimed area	5.5	Mulch and silt fences
Crandall Canyon	3.7-7A	Leach field	2.4	Mulch and silt fences
Crandall Canyon	3.7-7B	Diversion CCRD-22 area below Pond 014	0.1	Mulch and silt fences
Crandall Canyon	3.7-7B	Diversion CCRD-24 area	0.1	Mulch and silt fences

<sup>(a)</sup> Roads that do not drain to a sediment pond are excluded.

TABLE 7-10

STEEP SLOPE RIPRAP DESIGN VALUES

$D_{60}$ DETERMINED FROM DESIGN CURVE (FT)	DESIGN $D_{60}$ (FT)
< 0.25	0.25
0.26 - 0.50	0.50
0.51 - 0.75	0.75
0.76 - 1.00	1.00
1.01 - 1.25	1.25
1.26 - 1.50	1.50
1.51 - 1.75	1.75
1.76 - 2.00	2.00
2.01 - 2.25	2.25
2.26 - 2.50	2.50
2.51 - 2.75	2.75
2.76 - 3.00	3.00

Reference: Simons, Li and Associates, Inc., 1982.



TABLE 7-11

LIST OF WATER QUALITY PARAMETERS SAMPLED FROM APRIL 1977  
 THROUGH MAY 1978

<u>Field Parameters</u>		<u>Lab Parameters</u>
Water Level or Flow	Aluminum, total	Mercury, total
pH	Ammonia, as NH	Nitrate
Specific Conductance	Arsenic, total	Potassium, total
Temperature (Air & Water)	Barium, total	Phosphate, total
	Bicarbonate	Phosphate, ortho
	Boron, total	Oil and Grease
	Cadmium, total	Selenium, total
	Calcium, total	Silver, total
	Carbonate	Sodium, total
	Chemical Oxygen Demand	Specific Conductance
	Chloride, dissolved	Sulfate
	Chromium, total	Total Alkalinity
	Dissolved Oxygen	Total Dissolved Solids
	Fluoride, dissolved	Total Hardness
	Hydroxide	Total Organic Carbon
	Iron, total	Total Suspended Solids
	Lead, total	Turbidity
	Magnesium, total	Zinc, total
	Manganese, total	

TABLE 7-12  
LIST OF WATER QUALITY PARAMETERS SAMPLED FROM JUNE 1978  
THROUGH JUNE 1979

Field Parameters

Water Level or Flow

pH

Specific Conductance

Temperature (Air & Water)

Lab Parameters

Nitrite

Potassium, total

Phenols

Phosphate, ortho

Oil and Grease

Selenium, total

Silica

Silver, total

Sodium, total

Specific Conductance

Sulfate

Total Dissolved Solids

Total Organic Carbon

Total Suspended Solids

Zinc, total

Arsenic, total

Barium, total

Bicarbonate

Cadmium, total

Calcium, total

Chloride, dissolved

Chromium, total

Copper, total

Dissolved Oxygen

Fluoride, dissolved

Iron, total

Lead, total

Magnesium, total

Manganese, total

Nitrate

TABLE 7-13

LIST OF WATER QUALITY PARAMETERS SAMPLED FROM JUNE 1979  
 THROUGH DECEMBER 1979

<u>Field Parameters</u>		<u>Lab Parameters</u>
Water Level or Flow	Acidity	Nitrate
pH	Arsenic, total	Potassium, total
Specific Conductance	Barium, total	Phenols
Temperature (Air & Water)	Bicarbonate	Phosphate, ortho
	Cadmium, total	Phosphate, total
	Calcium, total	Oil and Grease
	Chloride, dissolved	Selenium, total
	Chromium, total	Silver, total
	Copper, total	Sodium, total
	Dissolved Oxygen	Specific Conductance
	Fluoride, dissolved	Sulfate
	Iron, total	Total Alkalinity
	Lead, total	Total Dissolved Solids
	Magnesium, total	Total Suspended Solids
	Manganese, total	Zinc, total

TABLE 7-14  
 LOCATION OF PRIOR AND EXISTING WATER MONITORING STATIONS IN  
 AND ADJACENT TO CASTLE GATE COAL COMPANY MINE PLAN AREA

<u>Location</u>		<u>Coordinates</u>
<b>Price River</b>		
B-5	Price River above Willow Creek	D-12-9-35 ddda
B-6	Price River above coal processing area	D-12-9-26 cdda
B-20	Price River above Sulfur Canyon Creek	D-12-9-22 ddca
<b>Ford Creek</b>		
B-21	Ford Creek at Highway U.S. 50 & 6	D-12-9-8 adcc
<b>Crandall Canyon</b>		
B-22	Spring one-half mile above Highway U.S. 50 & 6	D-12-9-27 bacc
B-22-1	Lower Crandall Canyon streambed	D-12-9-27 bacc
B-25	Crandall Canyon Creek above shaft area	D-12-9-28 dbdb
B-26	Crandall Canyon Creek below shaft area	D-12-9-28 dbda
B-43	Drill hole in Crandall Canyon	D-12-9-38 dbda
<b>Sulfur Canyon</b>		
B-19	Sulfur Canyon Creek above Price River	D-12-9-22 ddca
<b>Royal Mine</b> (Discontinued April 1978)		
BM-25	Water at bottom of Royal Mine	D-12-9-27 addb
BM-26	Royal Mine water 20 feet above bottom	D-12-9-27 addb
BM-27	Royal Mine water at cross cut #3	D-12-9-26 bccb
BM-28	Royal Mine water at 40 feet below top of mine	D-12-9-26 bcca
<b>Bear Canyon</b>		
BW-23	Upper well in Bear Canyon	D-12-9-34 bdcc
BW-24	Lower well in Bear Canyon	D-12-9-34 caac
B-42	Drill hole in Bear Canyon	D-12-9-33 daaa
<b>Adit No. 1</b>		
BM-30	Pipe down slope east of Highway 6	D-12-9-1-bbab

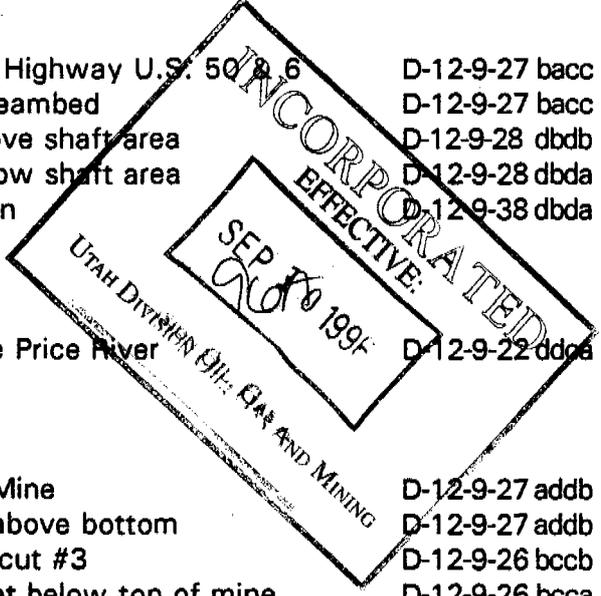
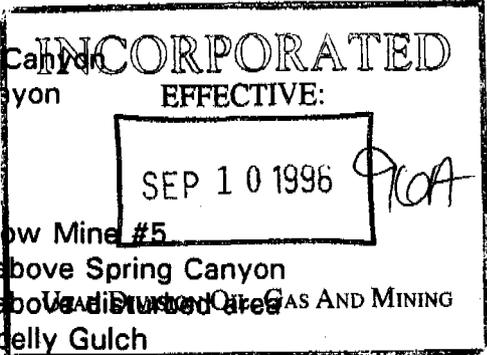


TABLE 7-14 (Continued)  
 LOCATION OF PRIOR AND EXISTING WATER MONITORING STATIONS IN  
 AND ADJACENT TO CASTLE GATE COAL COMPANY MINE PLAN AREA

<u>Location</u>		<u>Coordinates</u>
<b>Willow Creek</b>		
B-1	Upper Willow Creek	D-12-10-22 bbaa
B-2	Willow Creek above proposed Mine #6	D-12-10-31 acdd
B-3	Willow Creek below proposed Mine #6	D-13-10-6 bbba
<b>Hardscrabble Canyon</b>		
B-9	Holding pond below Mine #2 (Discontinued 5/78)	D-13-9-10 acbd
B-11	Hardscrabble below Mine #3	D-13-9-10 acdb
B-12	Hardscrabble above disturbed area	D-13-9-3 caac
<b>Springs</b>		
B-32	Spring in Mathis Canyon	D-12-10-27 aacd
B-33	Spring in Dry Canyon	D-13-10-3 bdcc
<b>Sowbelly Gulch</b>		
B-14	Holding pond below Mine #5	D-13-9-9 bbba
B-17	Sowbelly Gulch above Spring Canyon	D-13-9-16 abbb
B-28	Sowbelly Gulch above disturbed area	D-13-9-4 cbab
B-41	Drill hole in Sowbelly Gulch	D-13-9-5 aaba
B-45	Sowbelly Gulch below disturbed area by gate	D-13-9-9 bbdb
<b>Spring Canyon</b>		
BM-31	Near Mutual Spring - Old Portal	D-13-8-1 dcdd
B-32	Gravel Springs streambed	D-13-9-17 baab



Numbering System Used to Identify Springs and Wells in Utah

The numbers used in this report to identify springs and wells in Utah indicate the spring or well location by land subdivision according to a coordinate numbering system that was devised cooperatively by the Utah State Engineer and G.H. Taylor of the USGS about 1935. The system is illustrated in Figure 3 and on the following page. The coordinate number used to designate location of a well or spring comprises letters and numbers that designate consecutively the quadrant and township. A capital letter designating the quadrant in relation to the base point of the Salt Lake Base and Meridian; and numbers designating the township and range; the number of the section; the quarter section; the quarter of the quarter section; the quarter of the quarter-quarter section; and the particular spring or well within the 10-acre tract. For example D-3-2-34 bca refer to a spring or well in the NE 1/4 SW1/4 NW1/4 Section 34, Township 3 S, Range 2 E.

TABLE 7-14 (Continued)  
 LOCATION OF PRIOR AND EXISTING WATER MONITORING STATIONS IN  
 AND ADJACENT TO CASTLE GATE COAL COMPANY MINE PLAN AREA

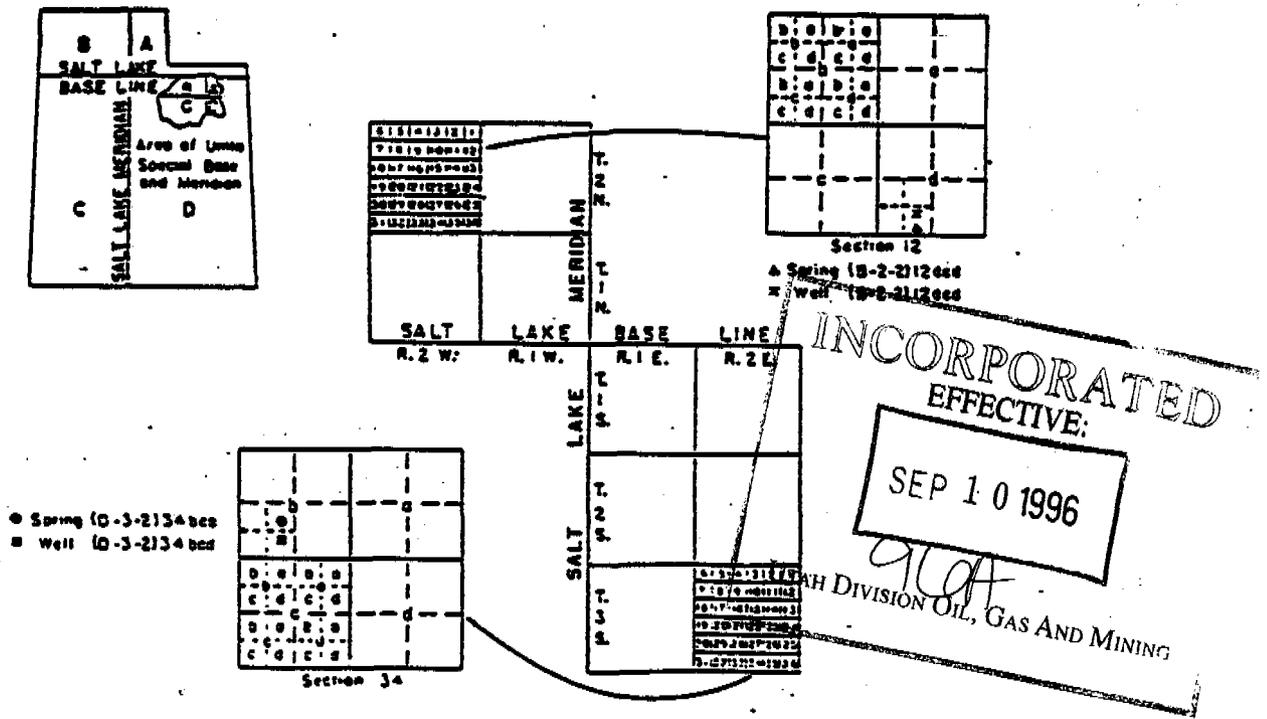


TABLE 7-15

LIST OF WATER QUALITY PARAMETERS MONITORED FROM JANUARY 1980  
THROUGH MARCH 1984

Field Parameters

Water Level or Flow  
pH  
Specific Conductance  
Temperature (Air & Water)

Parameters Monitored  
Intermittently  
as a Spot Check

Cyanide  
Iron, dissolved  
Mercury  
Nitrate  
Phosphate, ortho  
Silica  
Total Organic Carbon

Lab Parameters

Acidity  
Arsenic, total  
Barium, total  
Bicarbonate  
Cadmium, total  
Calcium, total  
Chloride, dissolved  
Chromium, total  
Copper, total  
Fluoride, dissolved  
Iron, total  
Lead, total  
Magnesium, total  
Manganese, total  
Nitrate  
Potassium, total  
Phenols  
Phosphate, total  
Oil and Grease  
Selenium, total  
Silver, total  
Sodium, total  
Sulfate  
Total Alkalinity  
Total Dissolved Solids  
Total Suspended Solids  
Zinc, total

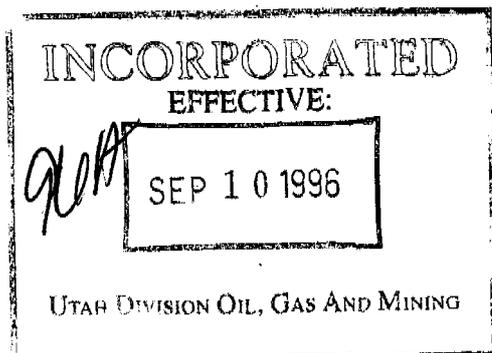


TABLE 7-16

LIST OF WATER QUALITY PARAMETERS FOR  
ACTIVE WATER MONITORING PROGRAM

Field parameters will include:

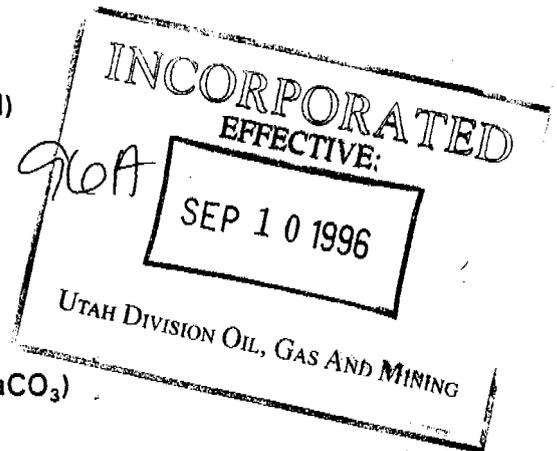
Flow (cfs)  
Temperature (°C)  
Spec. Conductance ( $\mu$ mhos/cm)  
pH (s.u.)  
Dissolved Oxygen (mg/l) (Perennial streams only)

Abbreviated Laboratory Analyses will include:

pH (s.u.)  
TDS (mg/l)  
TSS (mg/l)<sup>1</sup>  
Oil & Grease (mg/l)<sup>1,2</sup>  
Fe (mg/l)(dissolved and total)  
Ca (mg/l)  
Na (mg/l)  
K (mg/l)  
Cl (mg/l)  
Mg (mg/l)  
Bicarbonate (mg/l)  
Sulfate (mg/l)  
Total Hardness (mg/l) (as CaCO<sub>3</sub>)  
Carbonate (mg/l)  
Total Manganese (mg/l)  
Cation-Anion Balance (meq/l)

<sup>1</sup> Surface water samples only.

<sup>2</sup> Groundwater samples checked for visual signs.



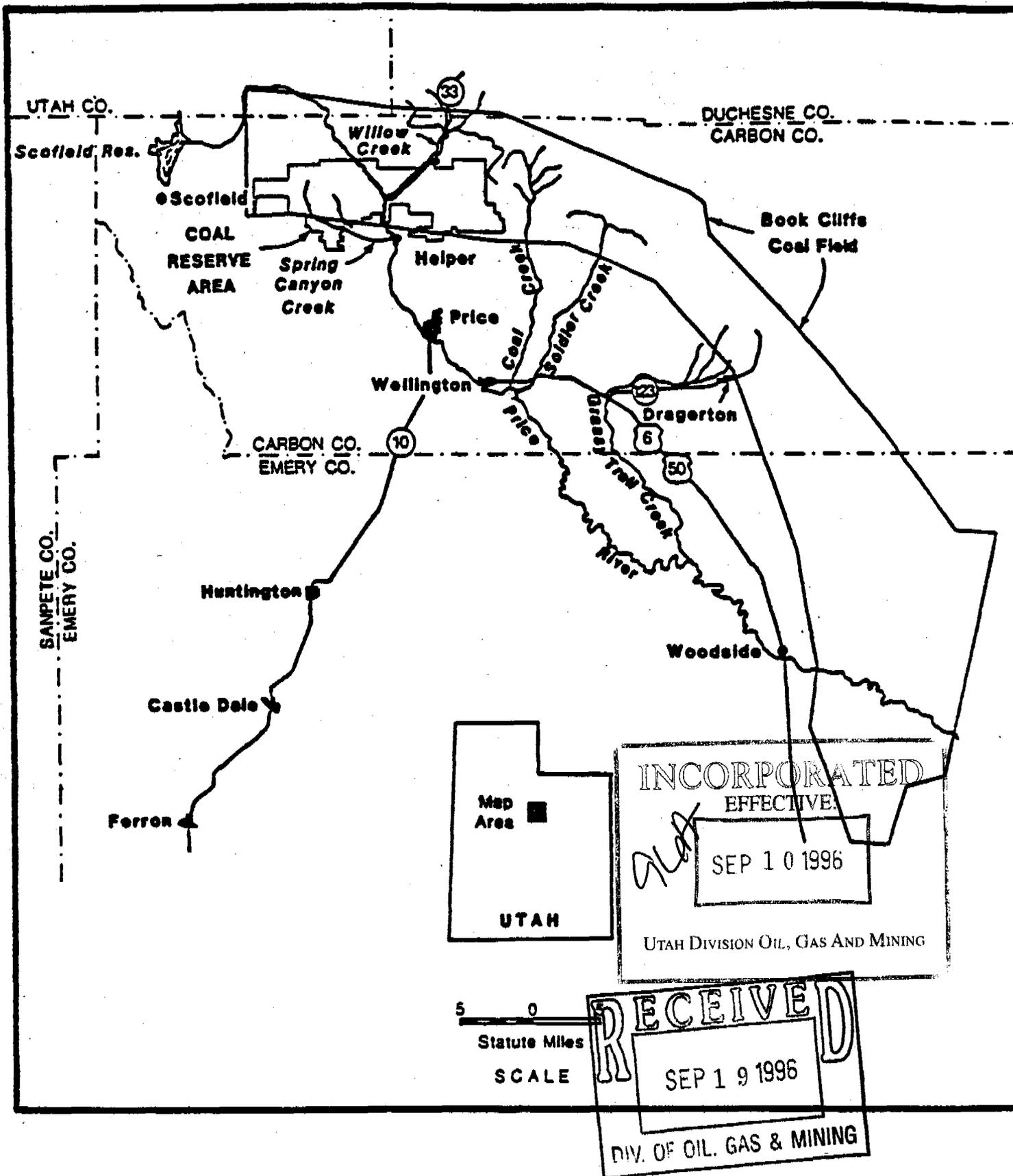


FIGURE 7-1. LOCATION OF THE PRICE RIVER COAL COMPANY MINE PLAN AREA.

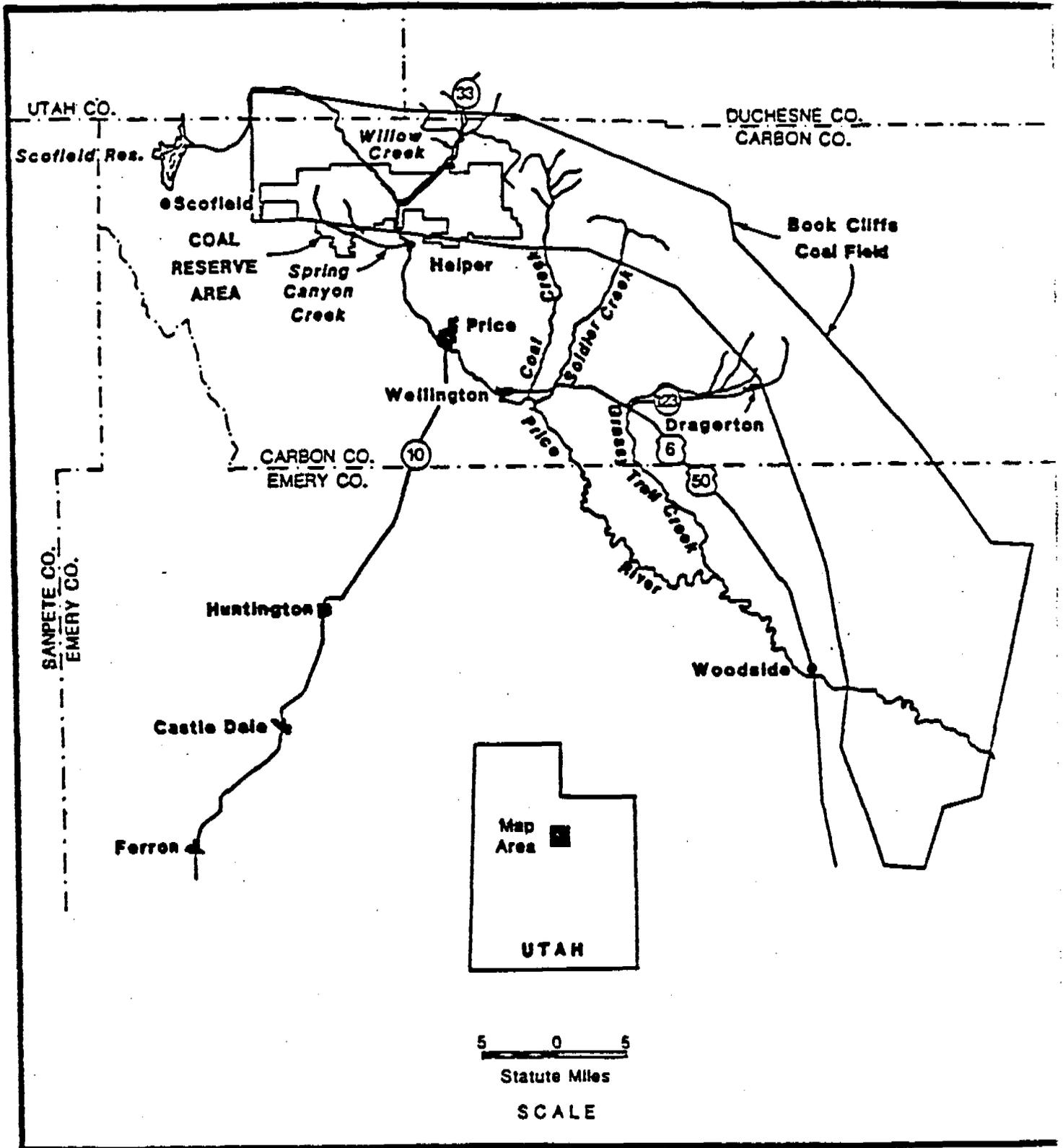
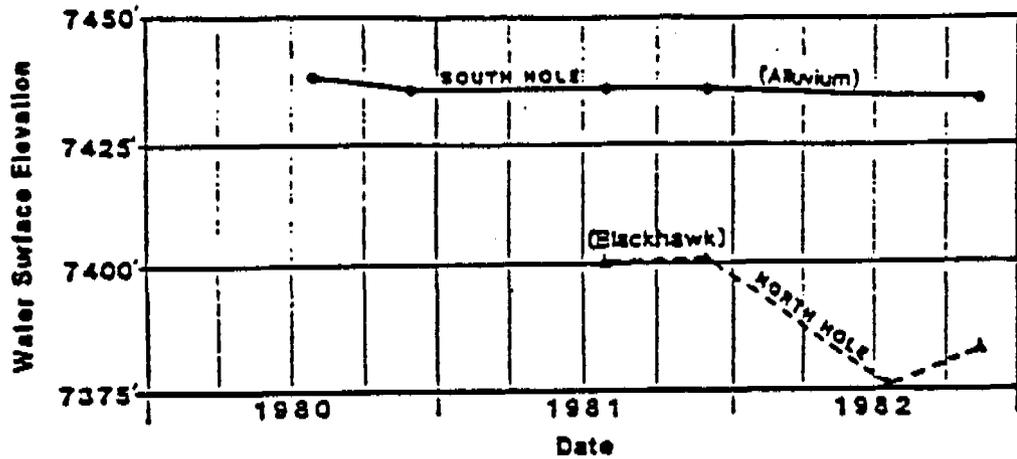
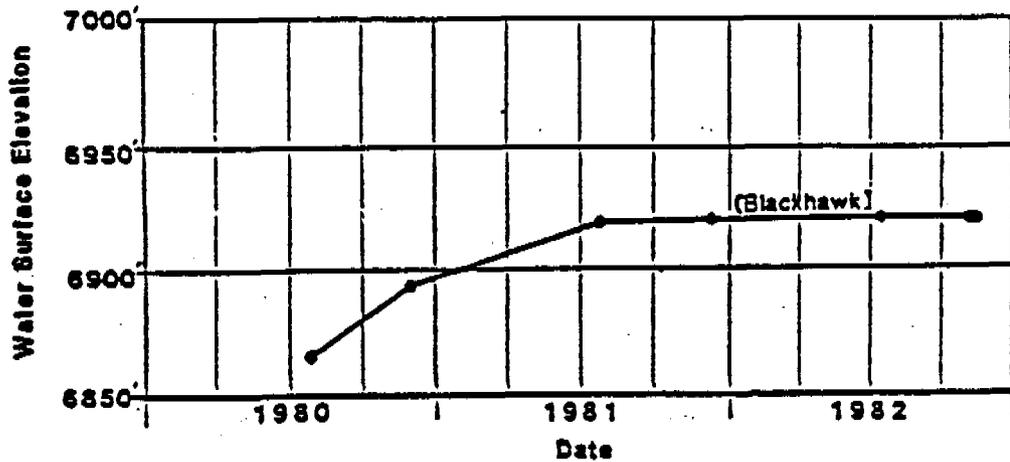


FIGURE 7-1. LOCATION OF THE PRICE RIVER COAL COMPANY MINE PLAN AREA.

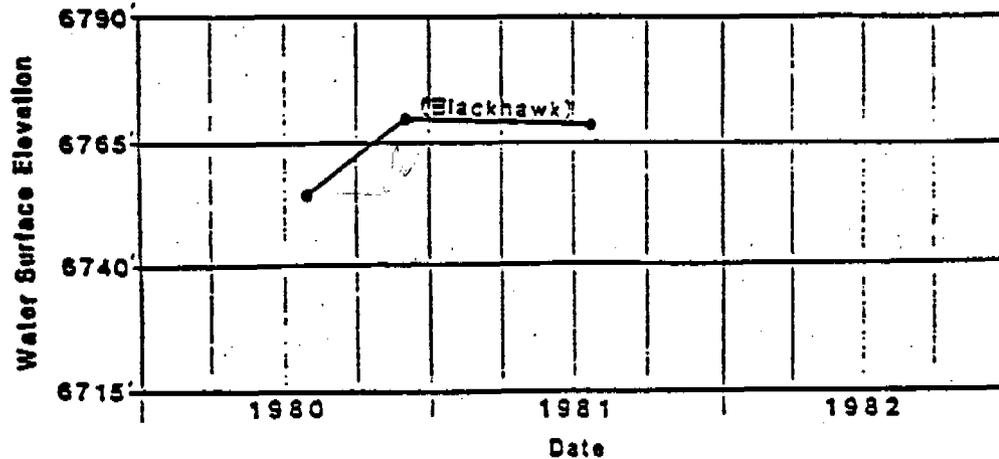
**MC 205 - Sowbelly Canyon**  
 Ground surface elevation - 7471.25'



**MC 206 - Bear Canyon**  
 Ground surface elevation - 7051.50'

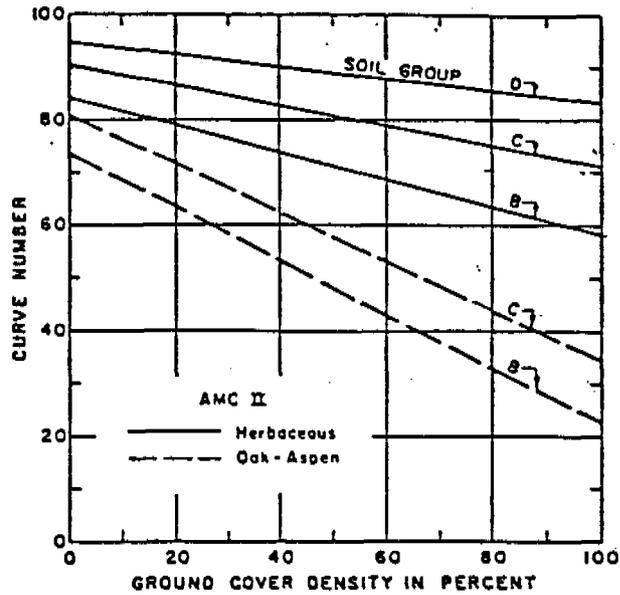


**MC 207 - Crandall Canyon**  
 Ground surface elevation - 6792.90'

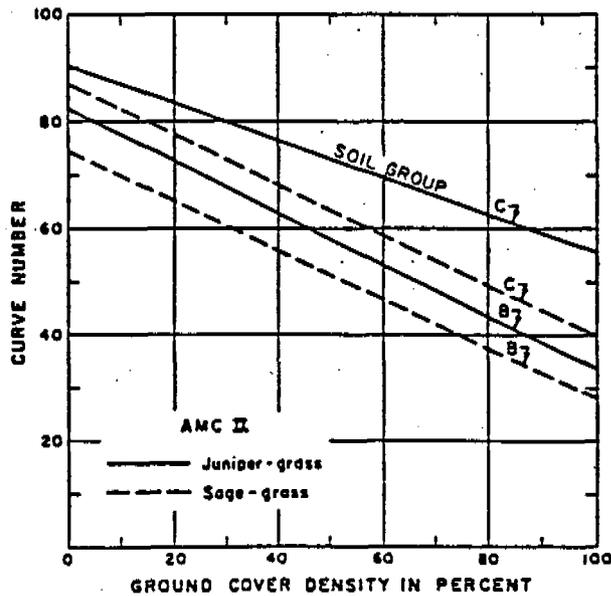


0793  
 0755  
 13

FIGURE 7-2. VARIATION IN WATER SURFACE ELEVATIONS IN OBSERVATION WELLS MC-205, MC-206, AND MC-207 SINCE 1980.



Graph for estimating runoff curve numbers of forest-range complexes in western United States: herbaceous and oak-aspen complexes.



Graph for estimating runoff curve numbers of forest-range complexes in western United States: juniper-grass and sage-grass complexes.

FIGURE 7-3. Curve Number Graphs (U.S. Soil Conservation Service, 1972).

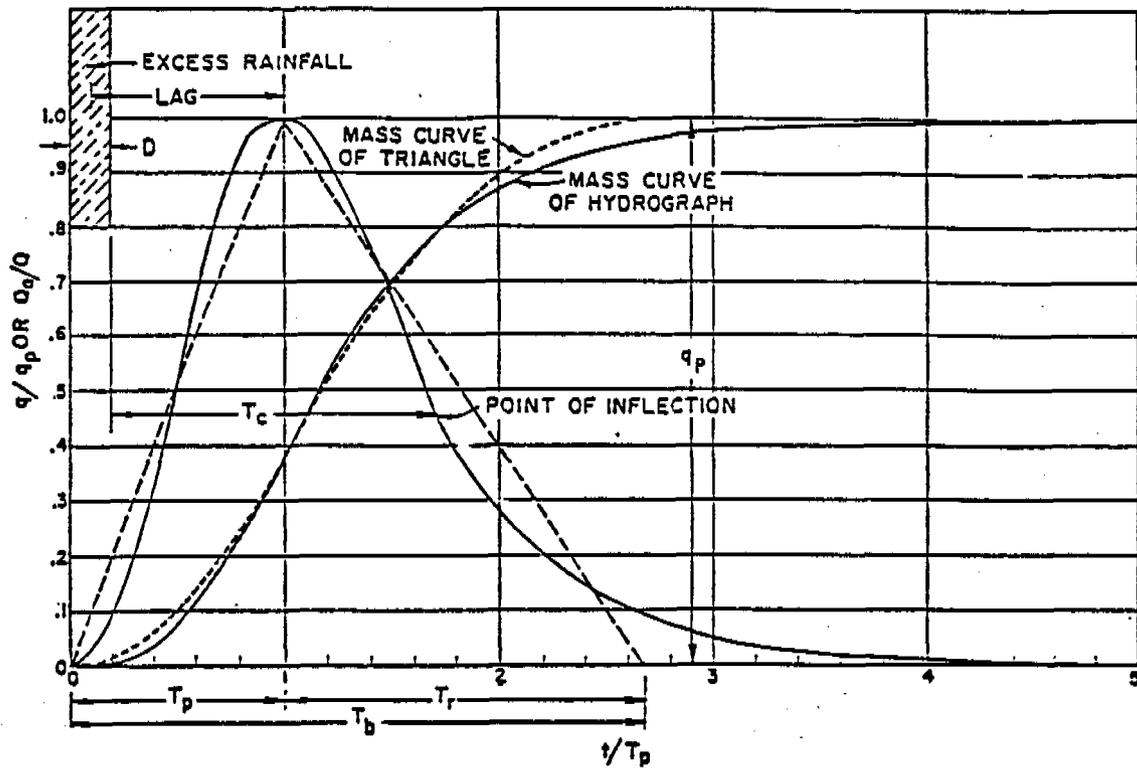


FIGURE 7-4. Curvilinear and Triangular Unit Hydrographs (U.S. Soil Conservation Service, 1972).

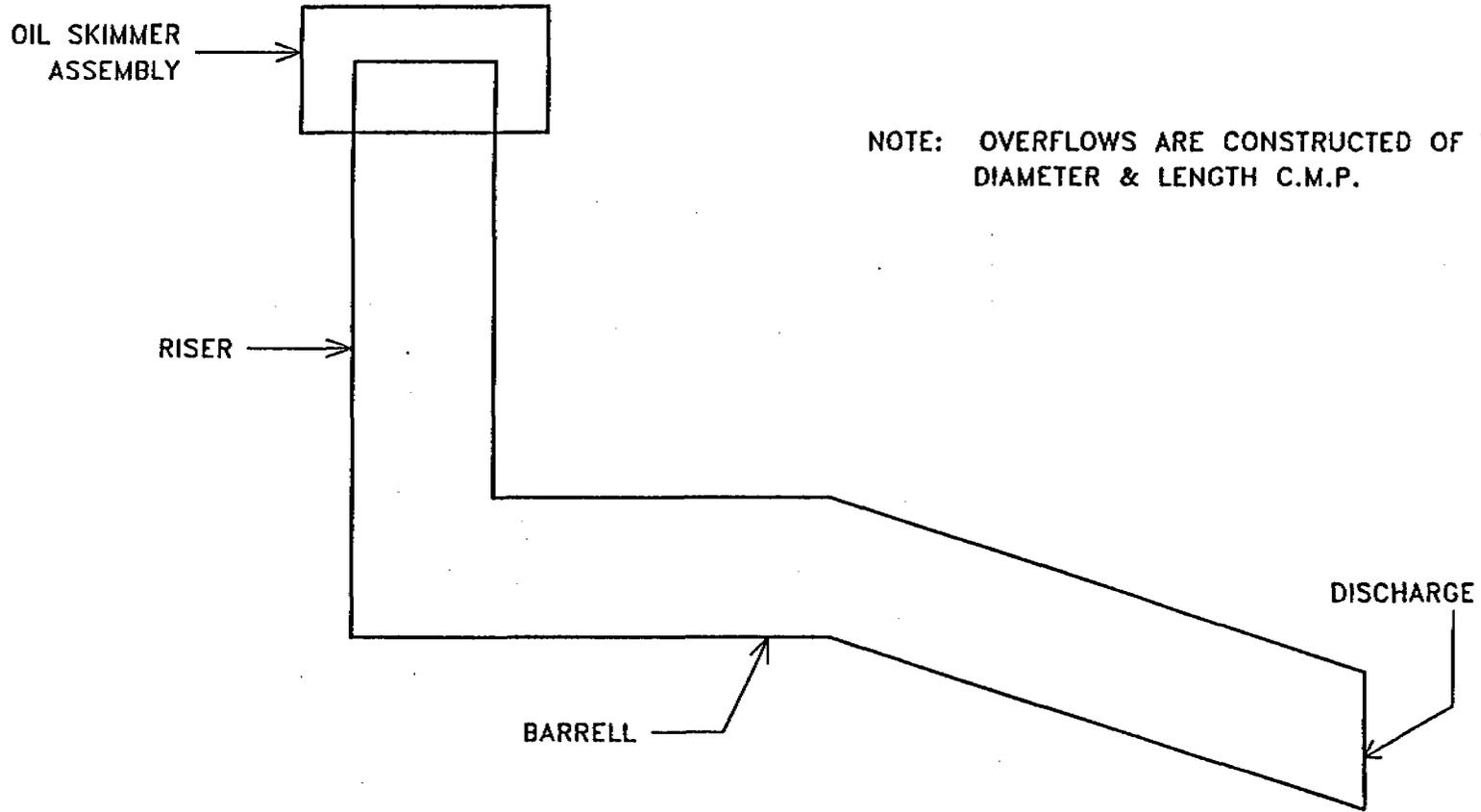


FIGURE 7-5. TYPICAL DROP-INLET OVERFLOW.

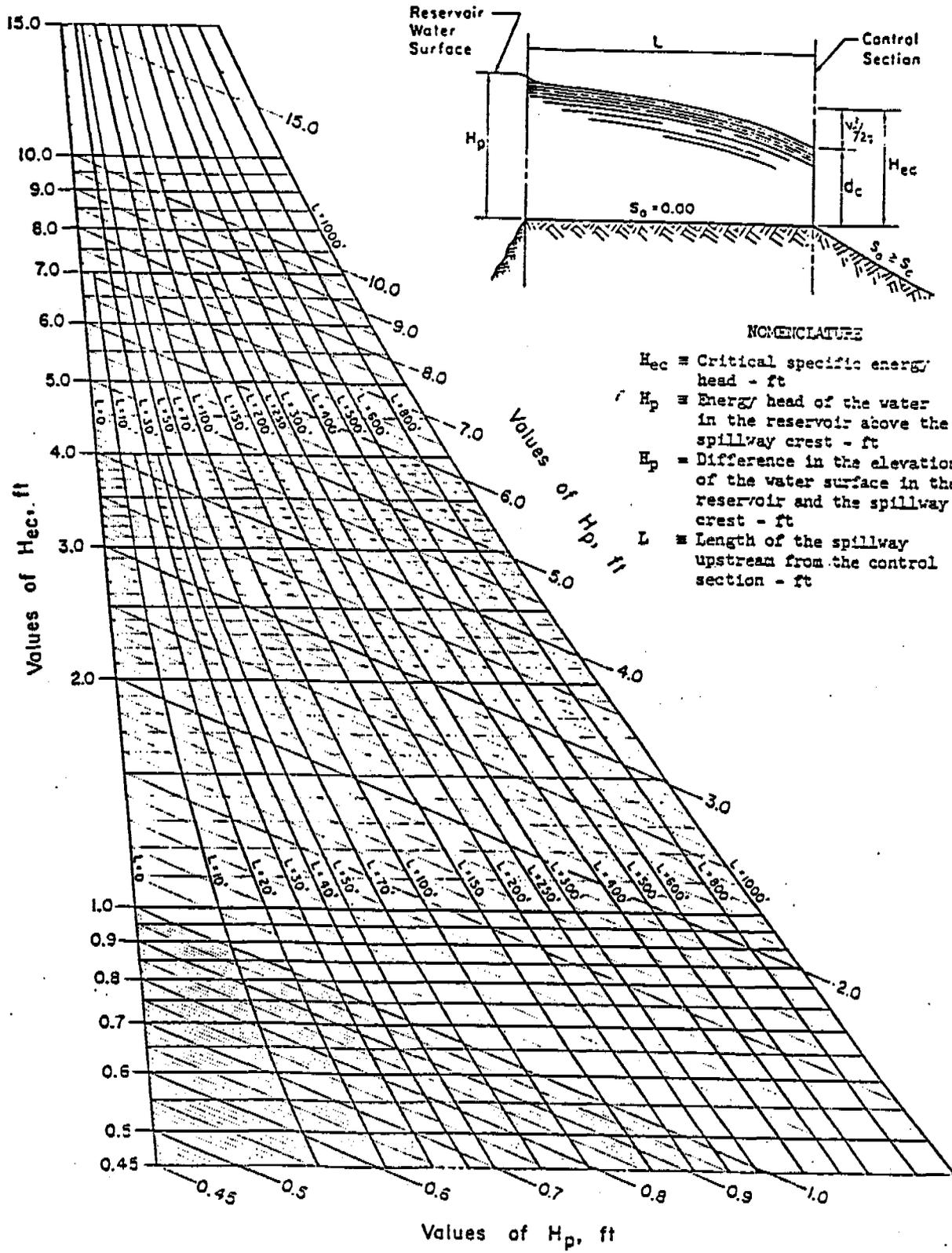


FIGURE 7-6. Head relationships for selected broad-crest weirs (U.S. Soil Conservation Service, 1968).

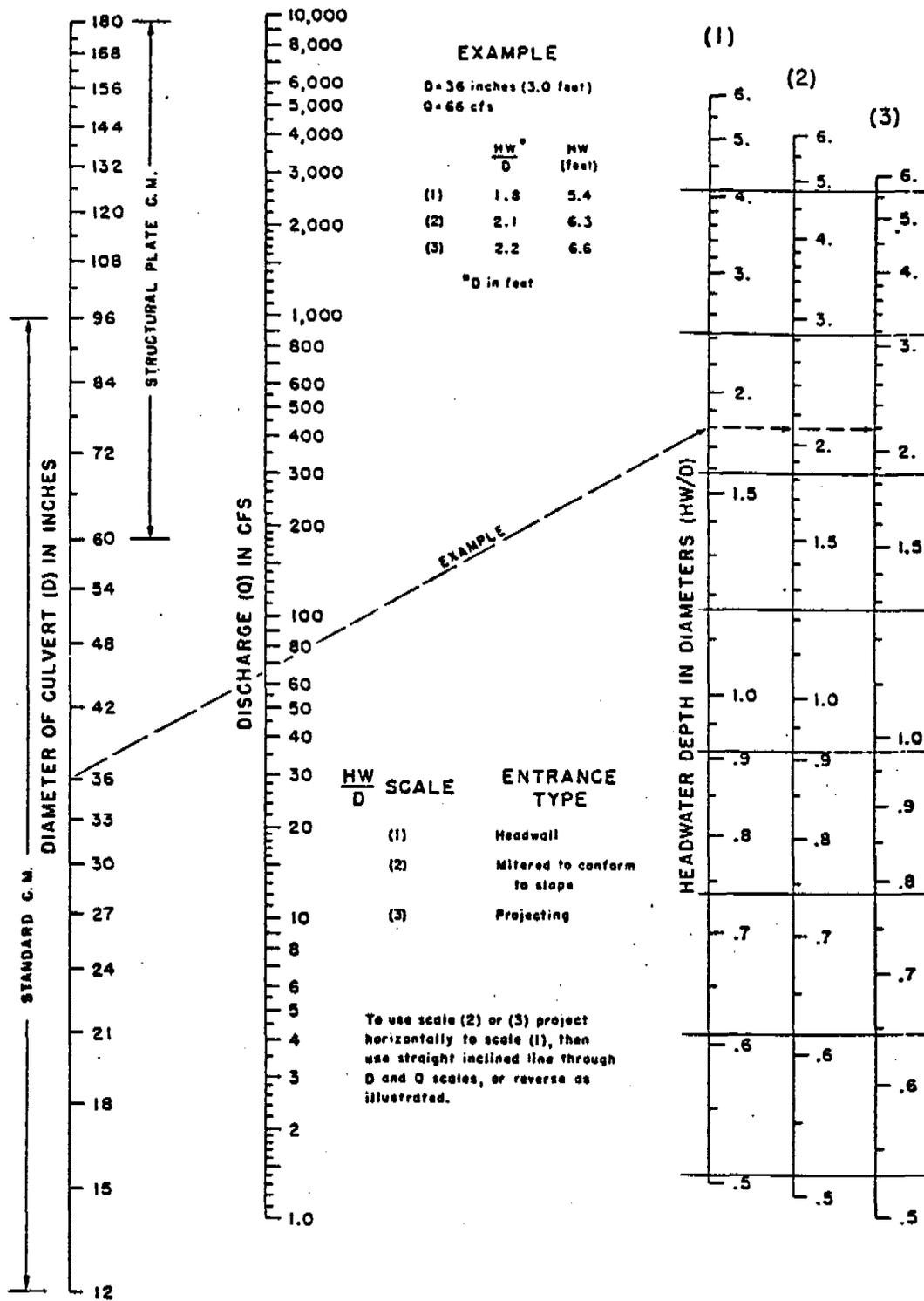


FIGURE 7-7. Headwater depth for C.M. Pipe Culverts with Inlet Control (U.S. Department of Transportation, 1977)

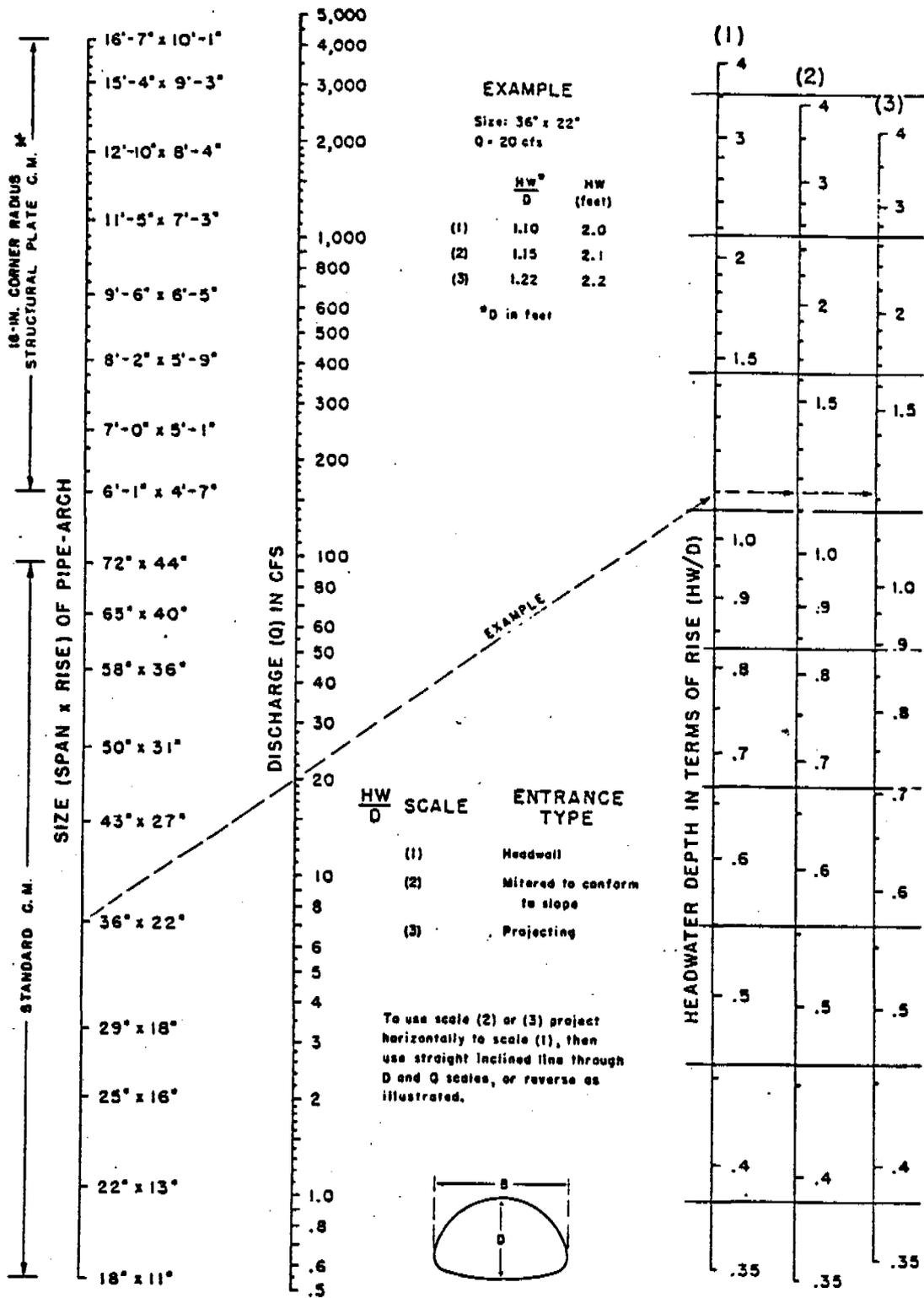


FIGURE 7-8. Headwater Depth for C.M. Pipe-Arch Culverts with Inlet Control (U.S. Department of Transportation, 1977).

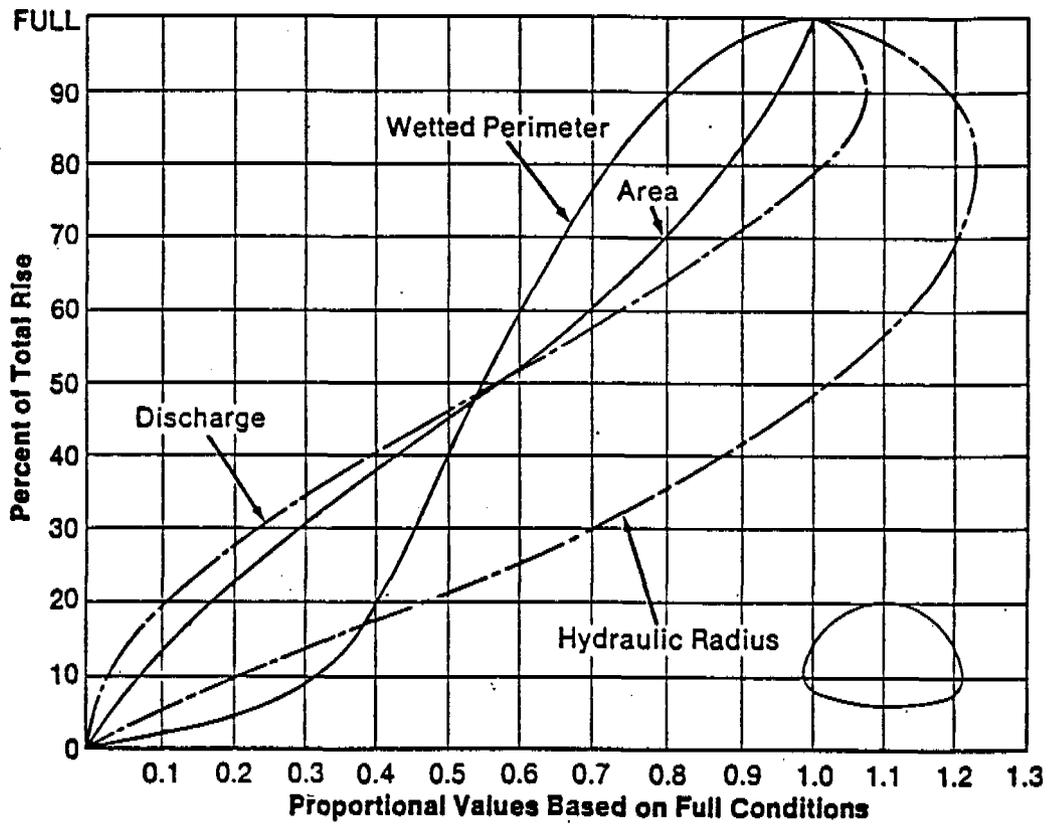


FIGURE 7-9. Hydraulic Properties of Corrugated Steel and Structural Plate Pipe-Arches (American Iron and Steel Institute, 1983).

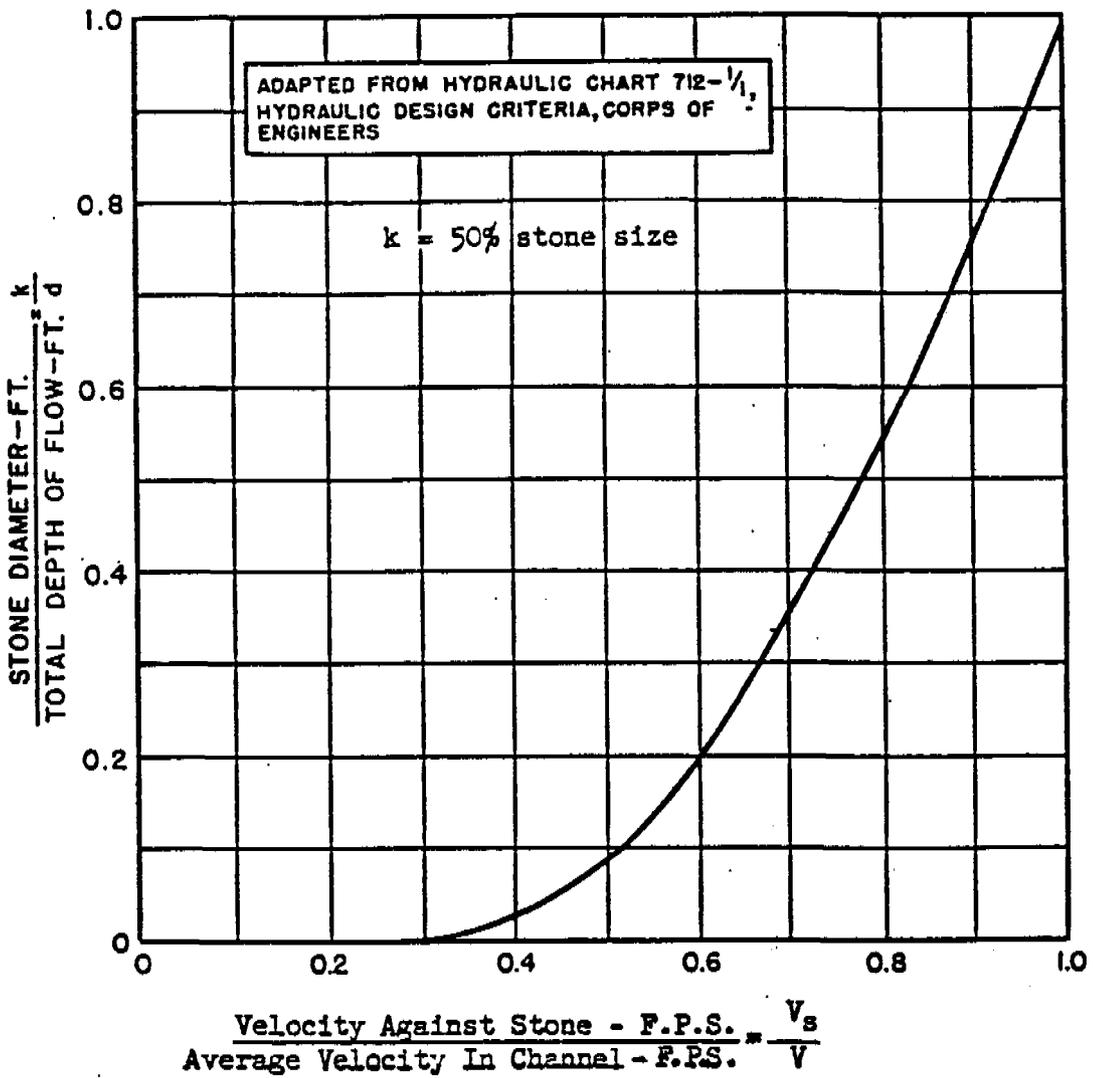


FIGURE 7-10a. Velocity Against Stone on Channel Bottom (U.S. Department of Transportation, 1978).

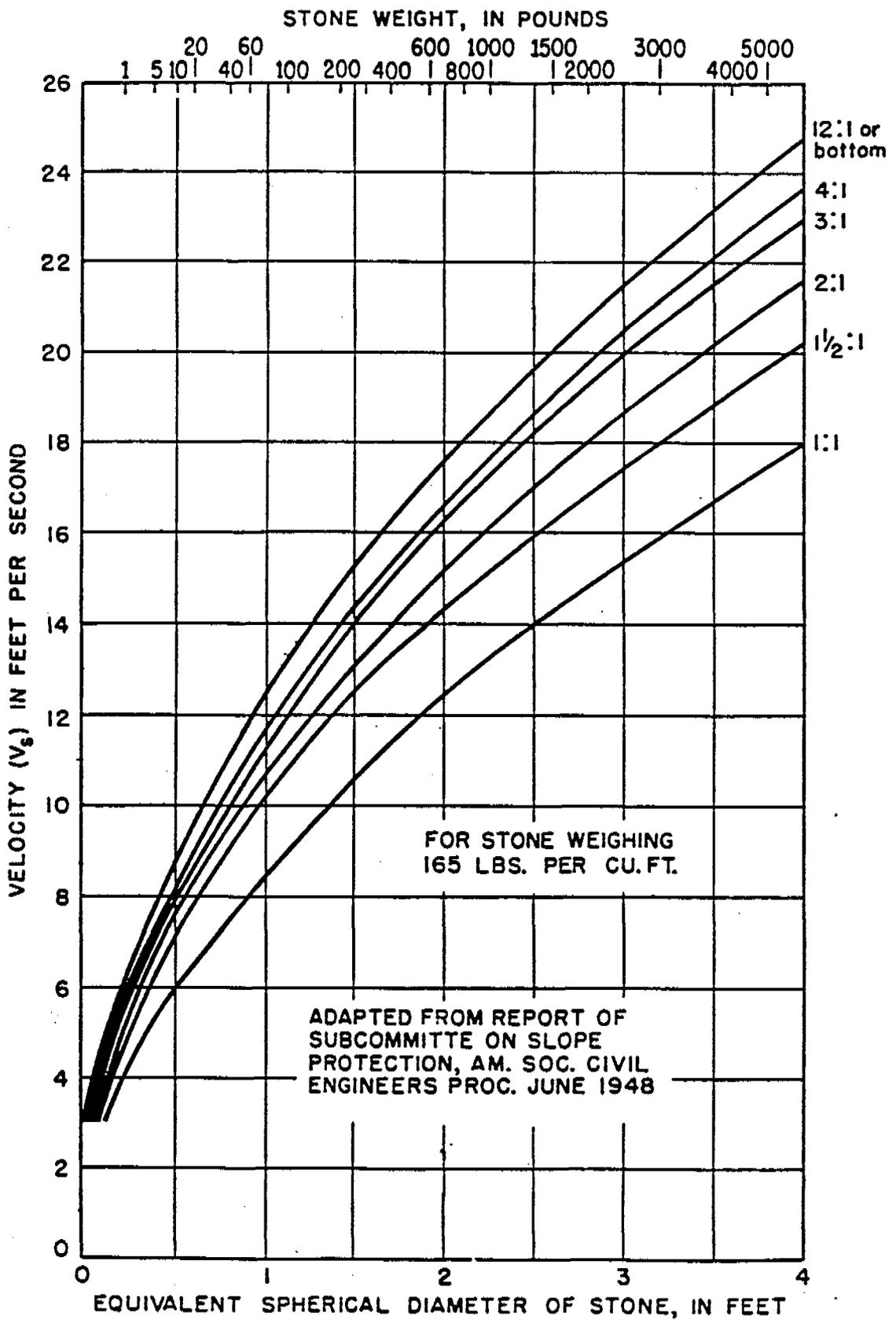
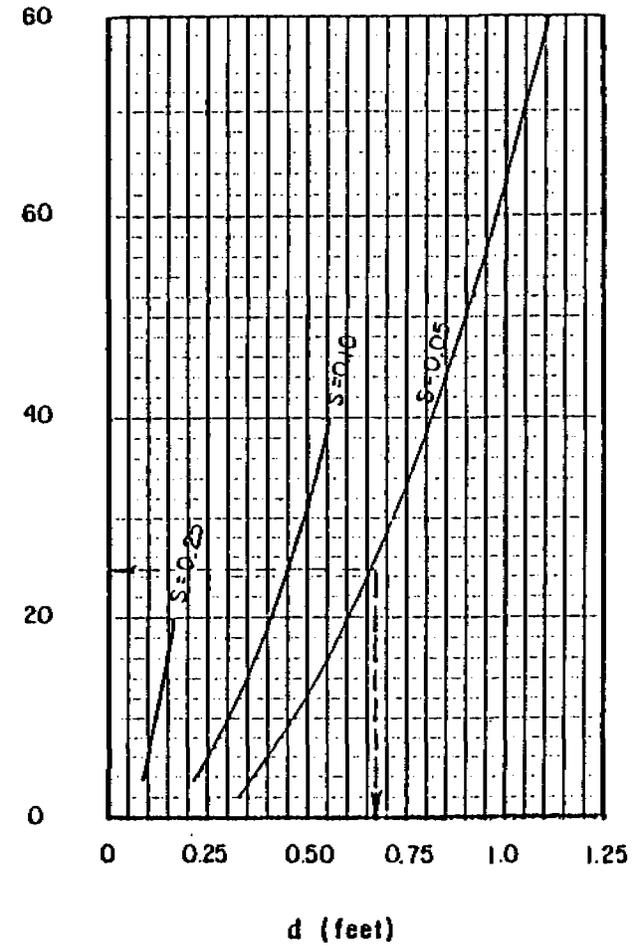
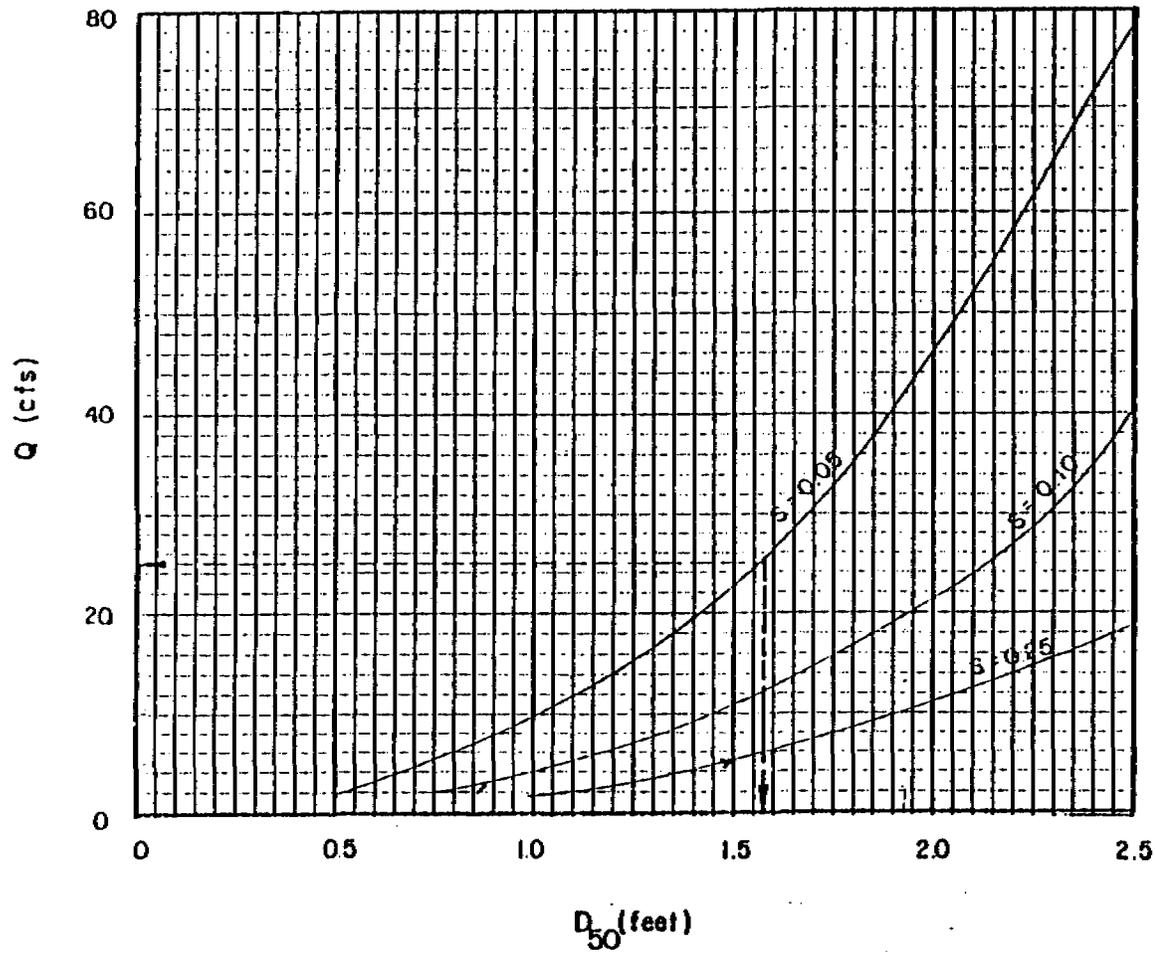


FIGURE 7-10b. Size of Stone that will Resist Displacement for Various Velocities and Side Slopes (U.S. Department of Transportation, 1978).

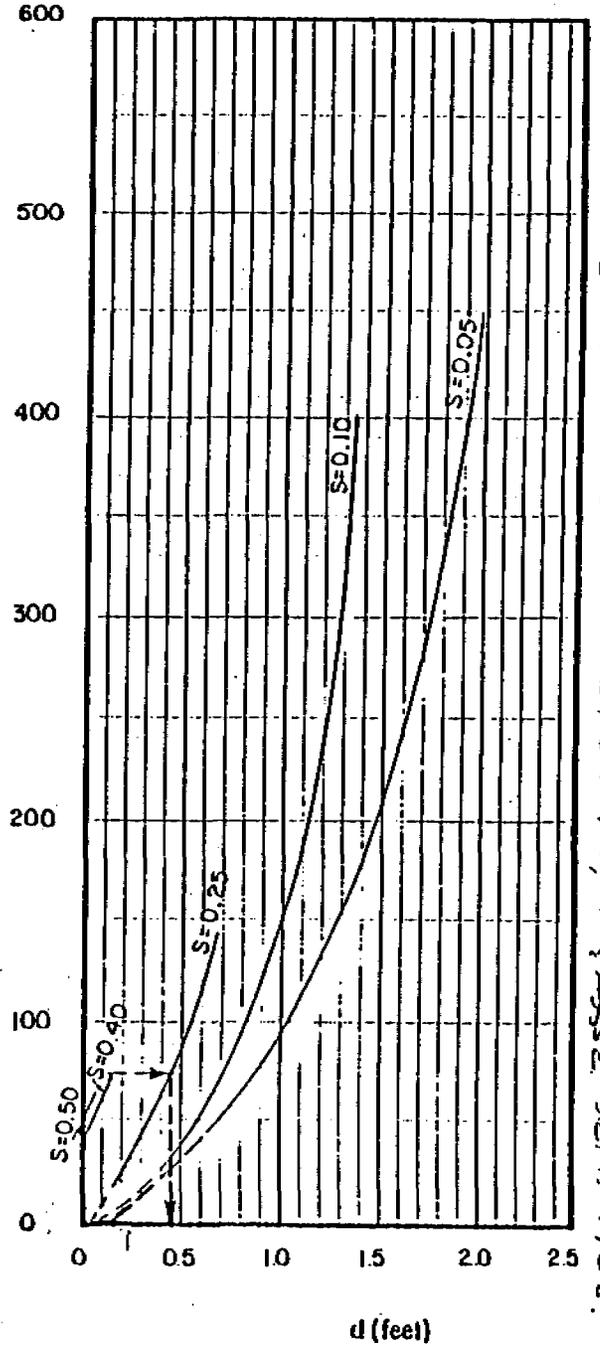
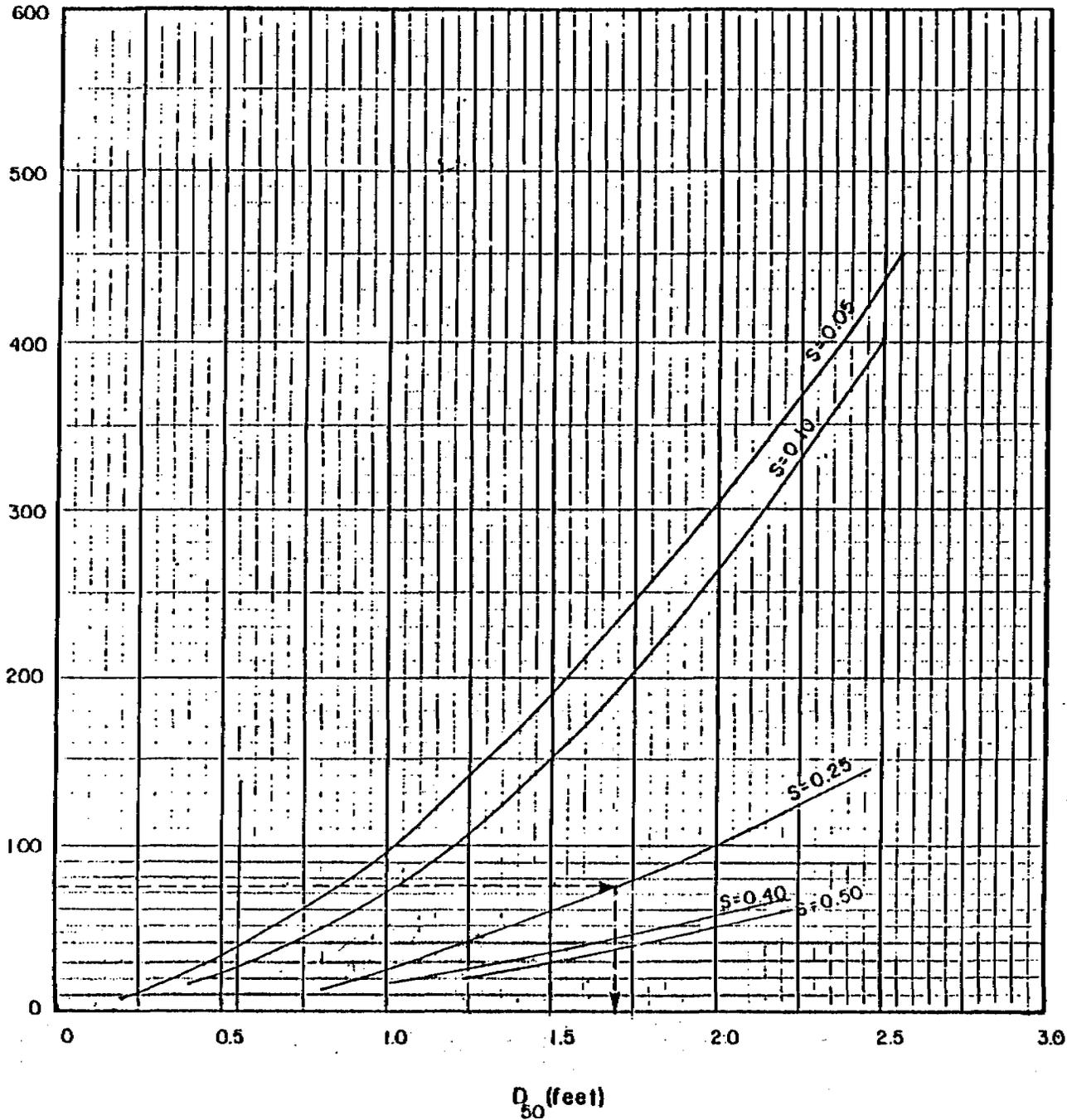


Reference: Simons, Li and Associates, Inc., 1982.

FIGURE 7-11a. STEEP-SLOPE RIPRAP DESIGN CURVES-CHANNEL BOTTOM WIDTH EQUALS ZERO.

007/004

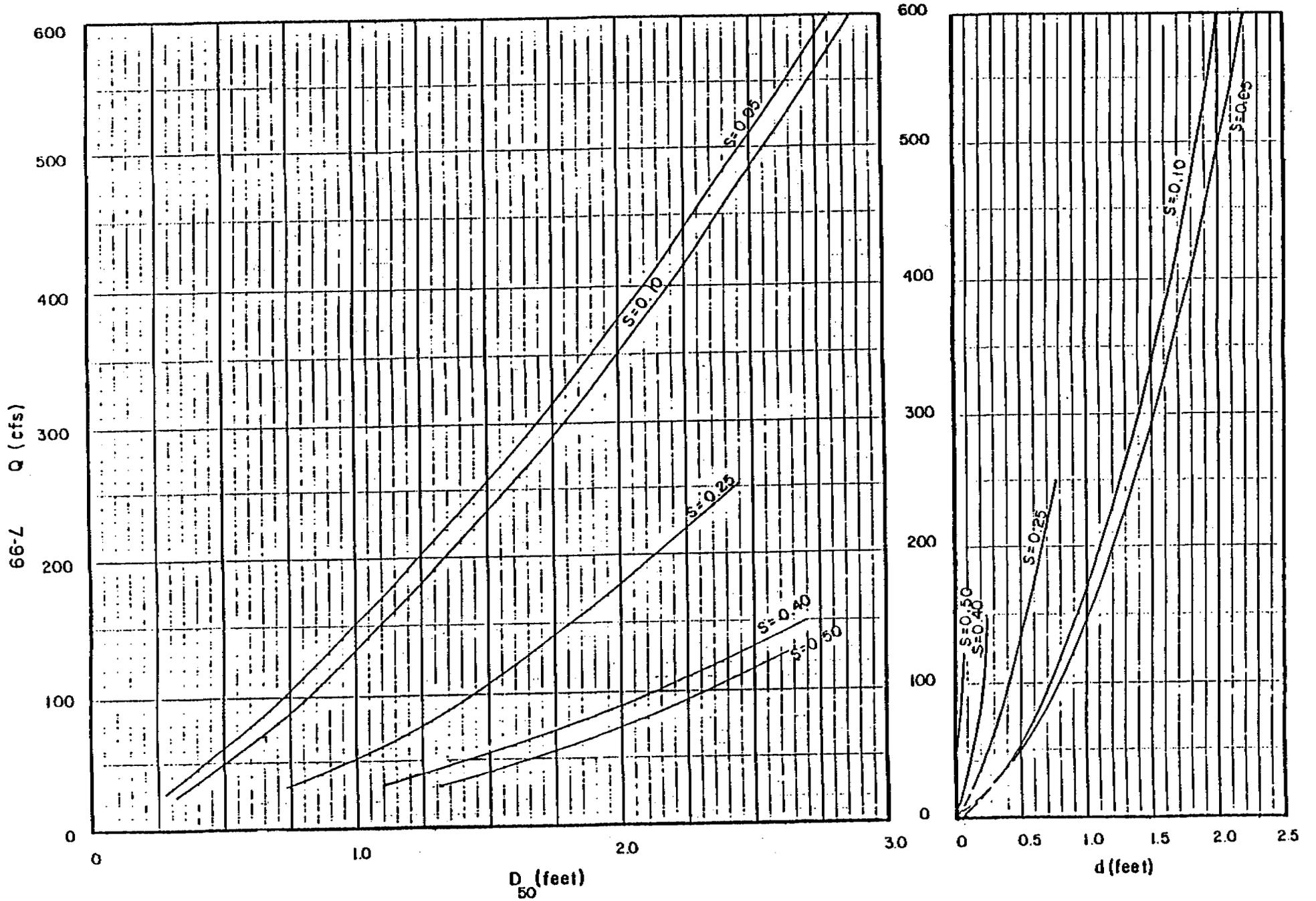
86-7 Q (cfs)



REF : OSM / TR - 82.2 SURFACE MINING WATER DIVISION DESIGN  
 MANUAL. TWEADED BY SIMONS, LI & ASSOC. SEPT. 1982.

Reference: Simons, Li and Associates, Inc., 1982.

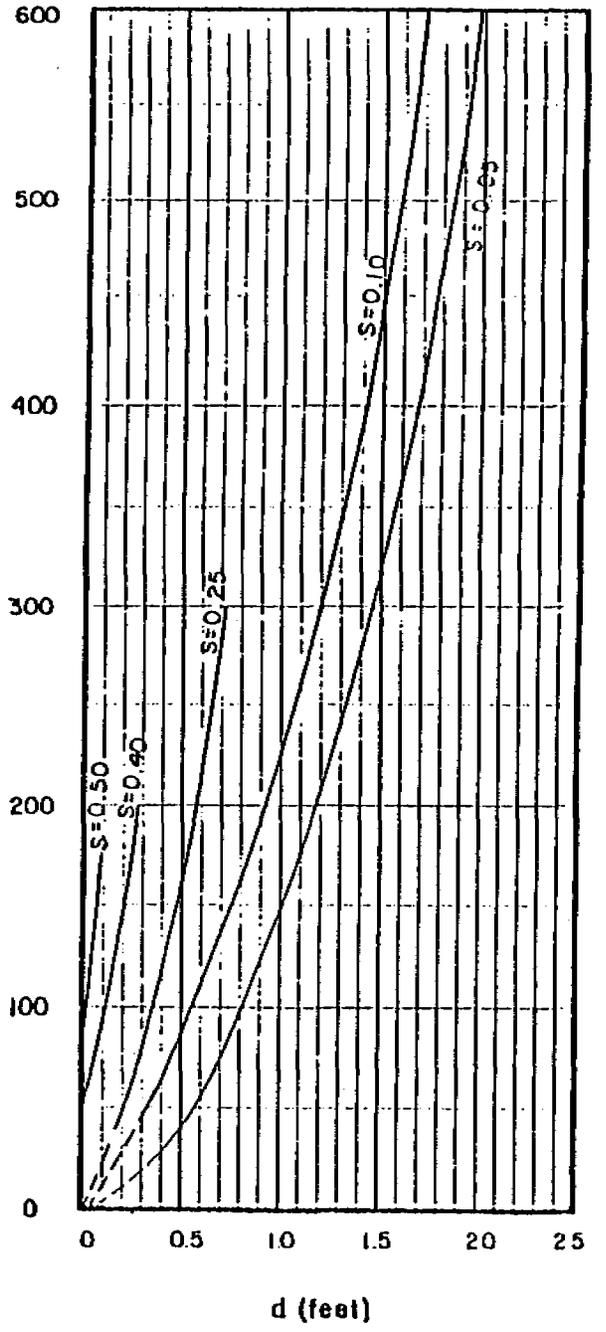
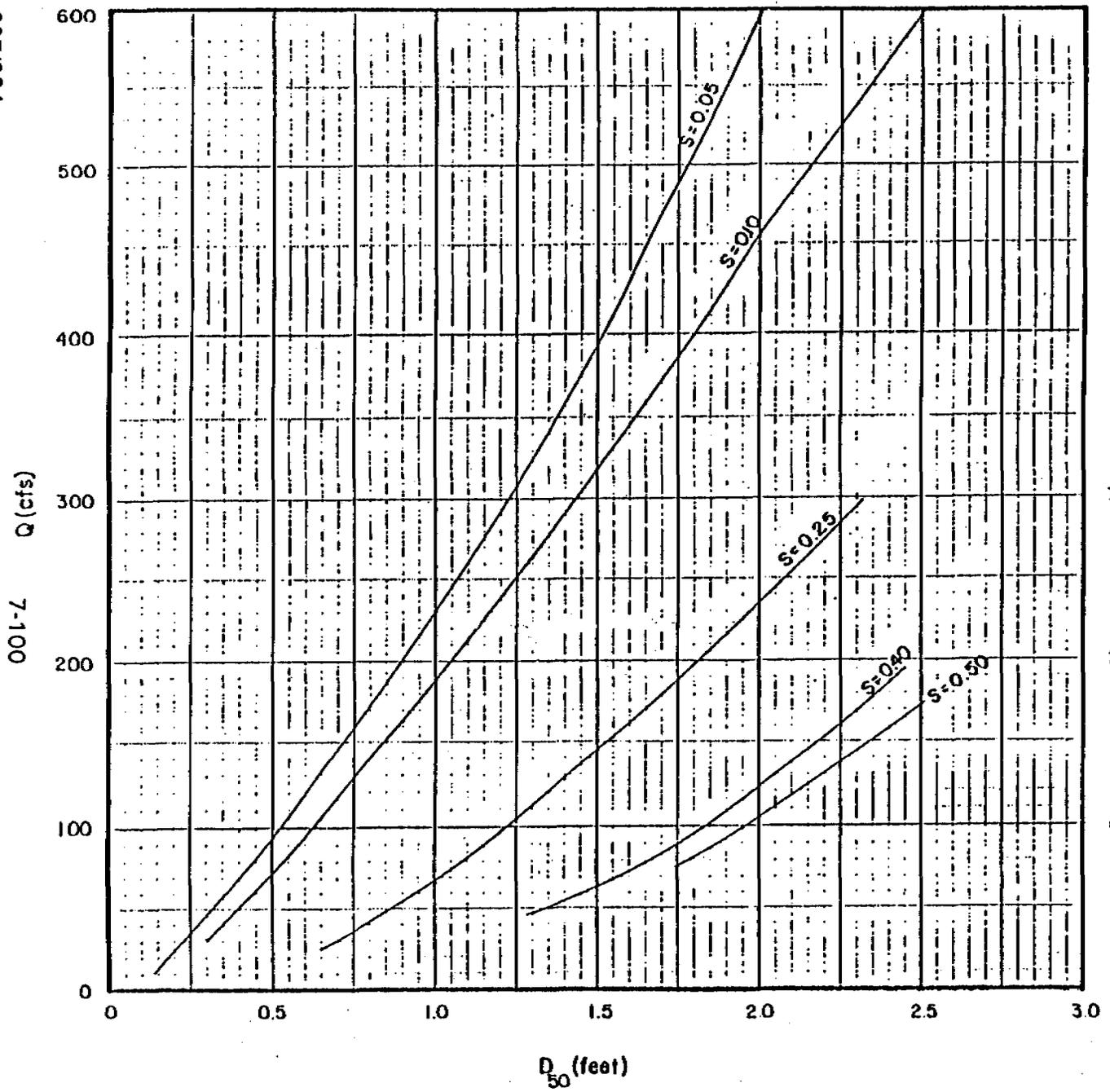
FIGURE 7-11b. STEEP-SLOPE RIPRAP DESIGN CURVES-CHANNEL BOTTOM WIDTH EQUALS SIX.



Reference: Simons, Li and Associates, Inc., 1982.

FIGURE 7-11c. STEEP-SLOPE RIPRAP DESIGN CURVES-CHANNEL BOTTOM WIDTH EQUALS TEN.

007/004

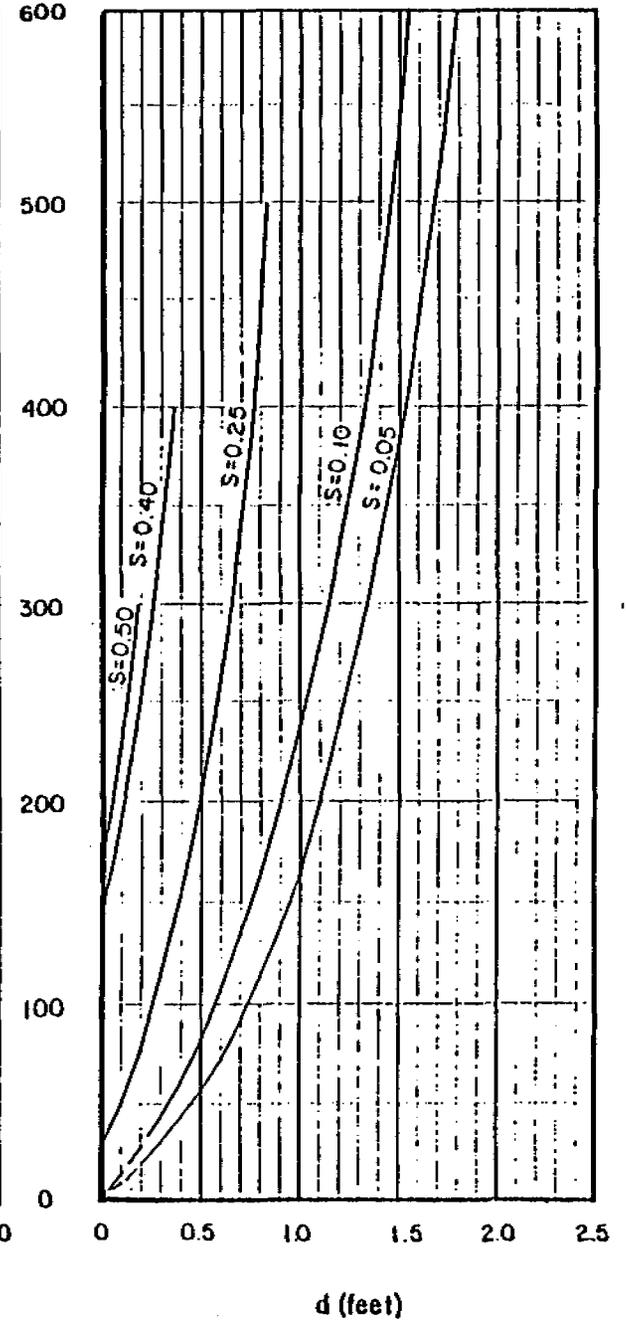
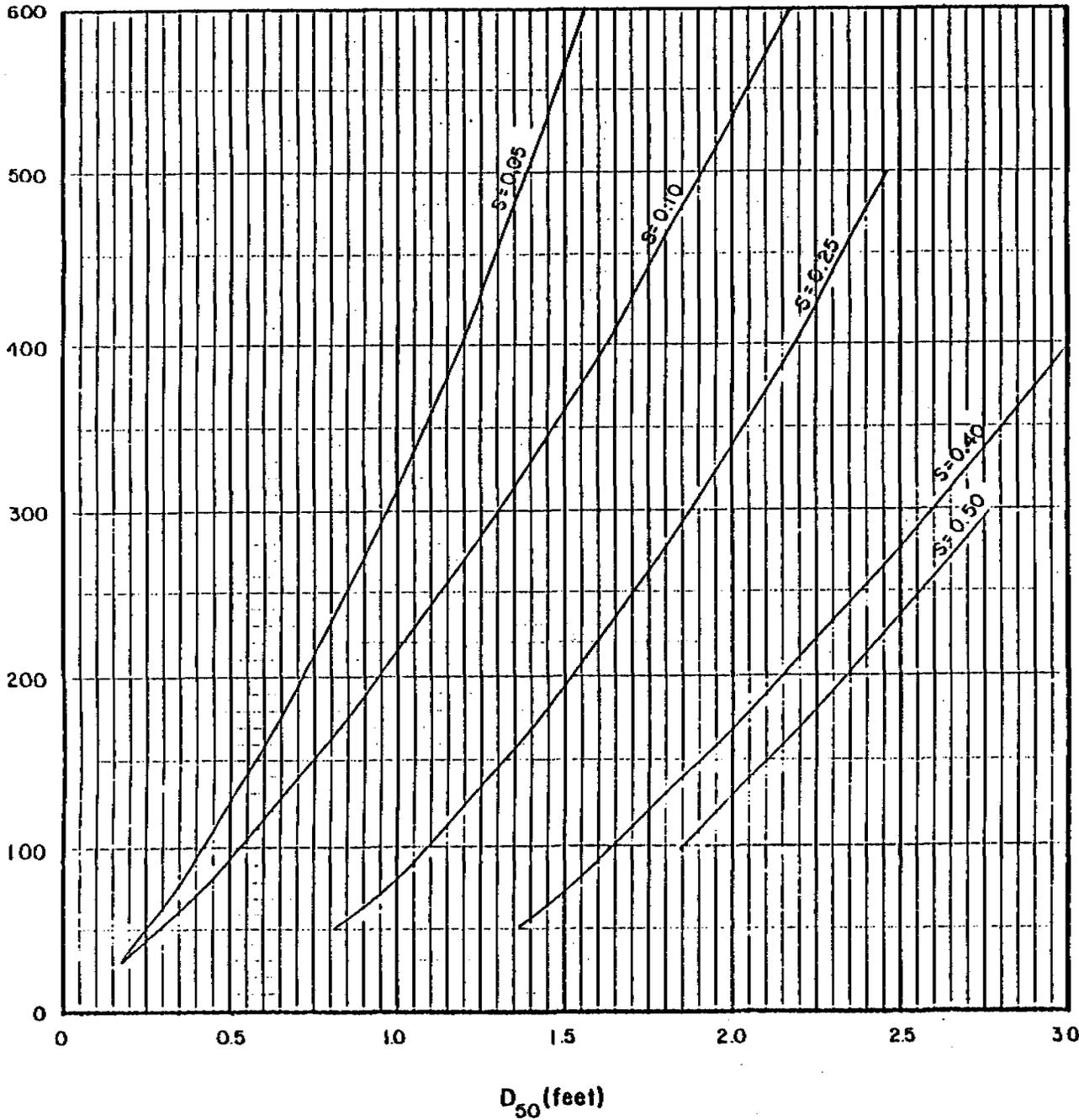


Reference: Simons, Li and Associates, Inc., 1982.

FIGURE 7-11d. STEEP-SLOPE RIPRAP DESIGN CURVES-CHANNEL BOTTOM WIDTH EQUALS 14.

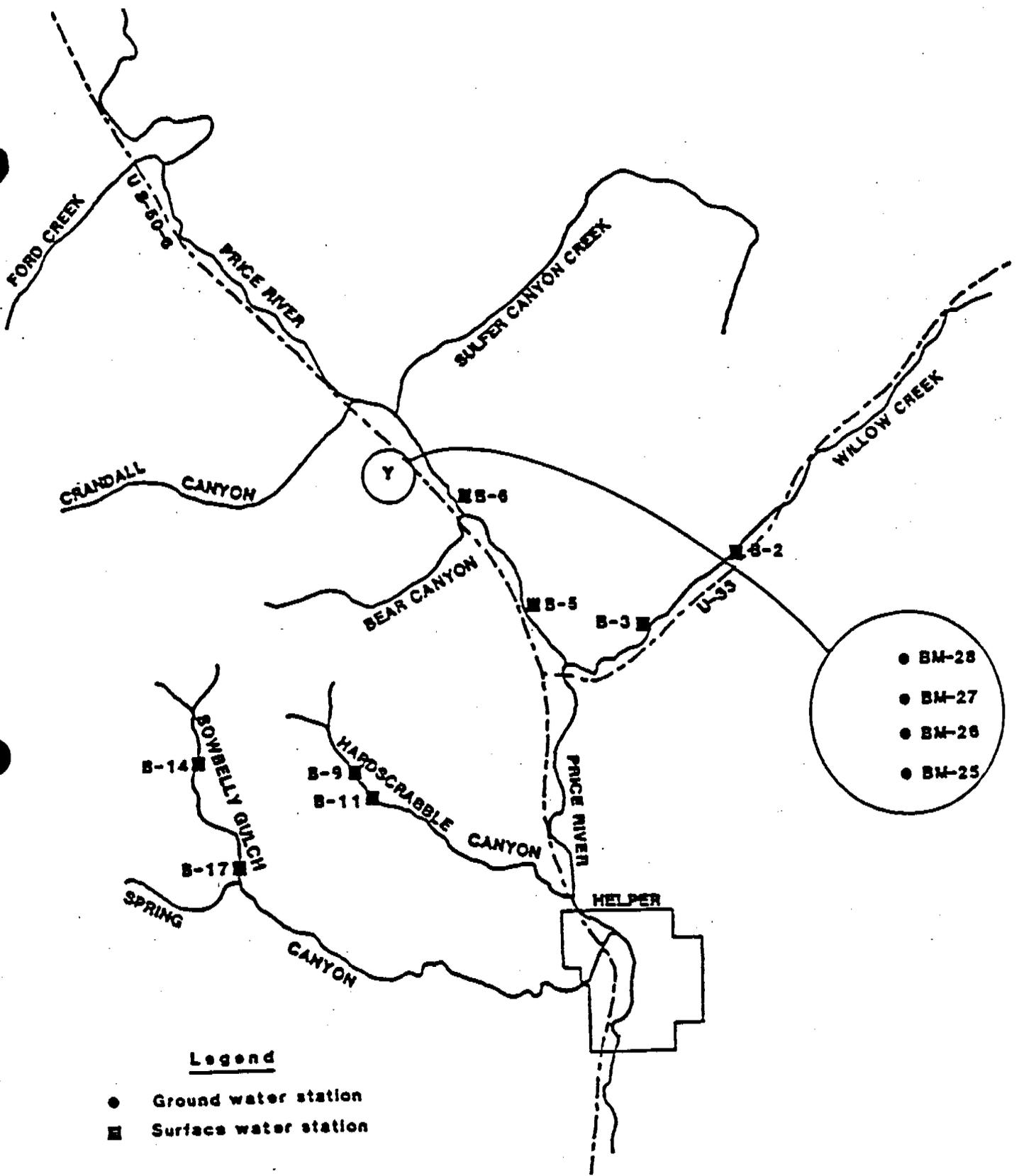
007/004

101-7 Q (cfs)  
7-101



Reference: Simons, Li and Associates, Inc., 1982.

FIGURE 7-11e. STEEP-SLOPE RIPRAP DESIGN CURVES-CHANNEL BOTTOM WIDTH EQUALS 20.



**Legend**

- Ground water station
- Surface water station

FIGURE 7-12. SAMPLING STATION LOCATIONS FOR PRICE RIVER COAL COMPANY MONITORING PROGRAM FROM APRIL, 1977 THROUGH JUNE, 1978.

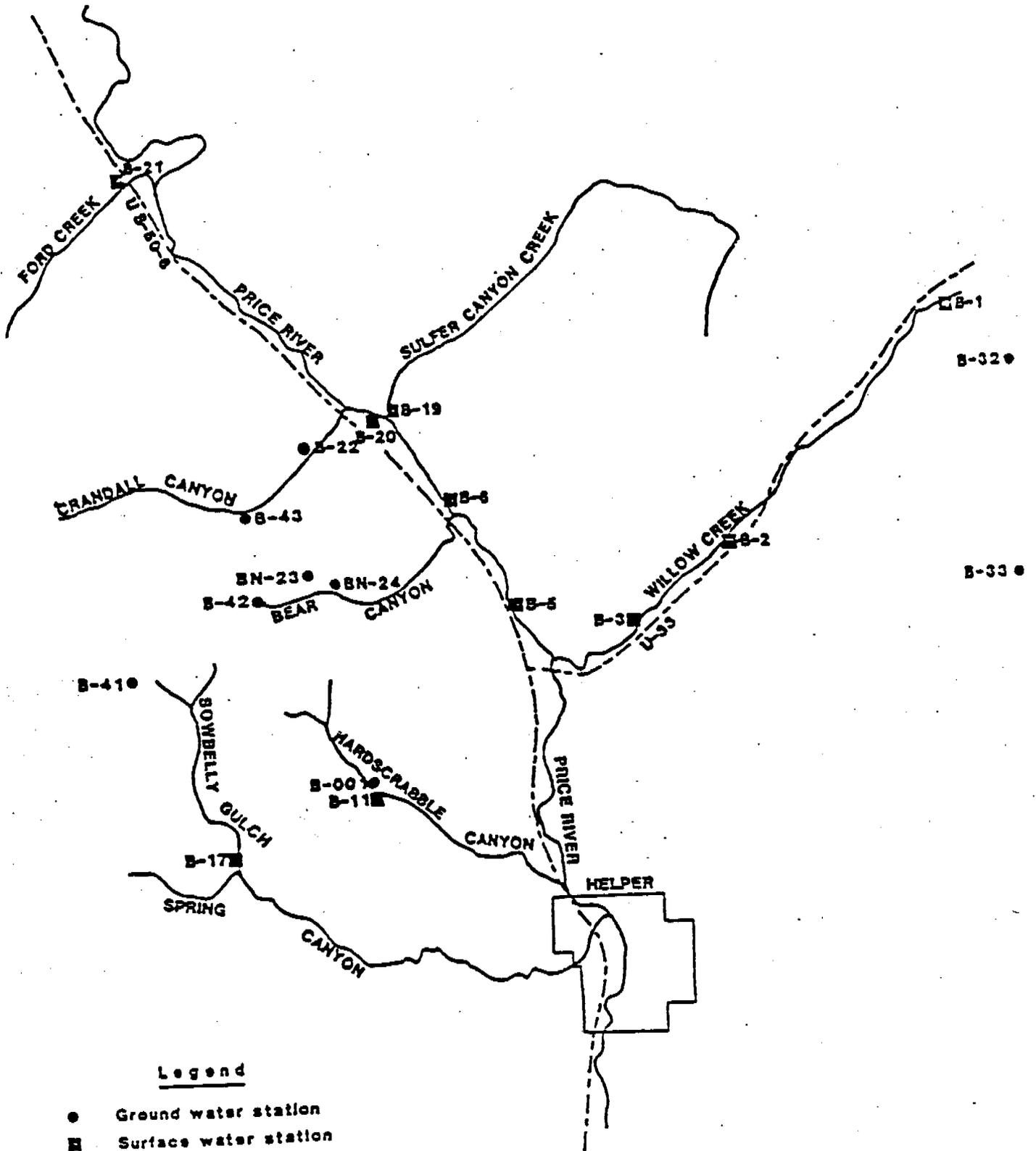


FIGURE 7-13. WATER SAMPLING STATION LOCATIONS FOR PRICE RIVER COAL COMPANY FROM JUNE, 1978 THROUGH DECEMBER, 1979.

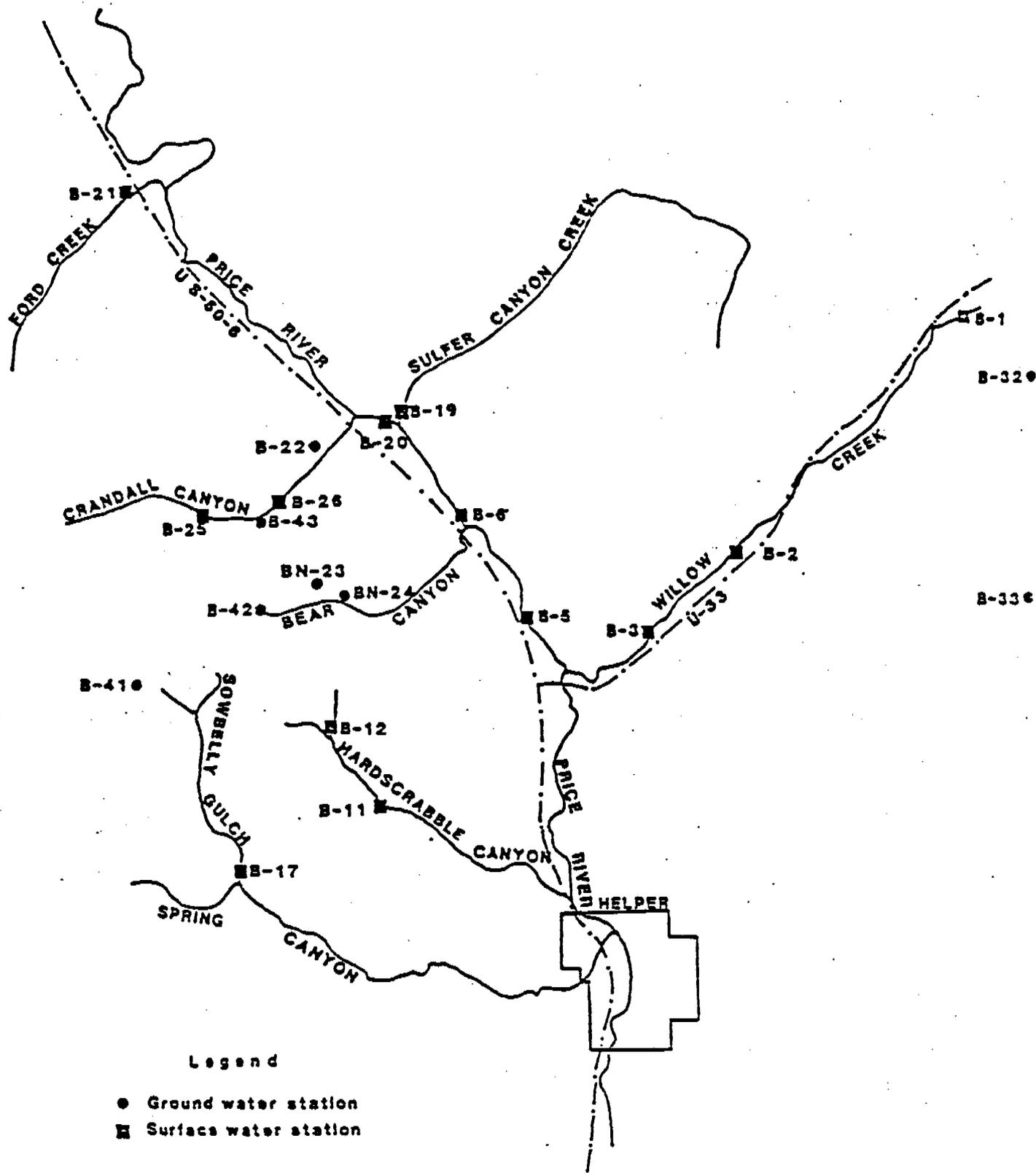


FIGURE 7-14. WATER SAMPLING STATION LOCATIONS FOR PRICE RIVER COAL COMPANY FROM JANUARY, 1980 THROUGH MARCH, 1984.

**APPENDIX 7-1**  
**AQUIFER TESTING PROCEDURES**

## Hydraulic Conductivity Tests

As outlined in a January 20, 1983, letter from Golder Associates, Inc. of Seattle, Washington to Robert Wiley of Price River Coal, the procedures followed by Golder Associates in conducting hydraulic conductivity tests on test wells MC-205, MC-206, and MC-207 were as follows:

The hydraulic conductivities of the in-situ bedrock materials were obtained from a drilling program by carrying out falling-head permeability tests using through-the-bit inflatable packers. Typically, the tests were carried out in 50-foot long zones at principal strata changes throughout the drillhole stratigraphy.

1. The drill string is backed off from the bottom of the hole to the top of the interval to be tested;
2. A straddle packer system is attached to a wireline cable and placed down the drill rod such that the lower packer protrudes below the bit while the upper packer remains in the drill rod;
3. The packers are inflated pneumatically. The test interval is now insulated from the rest of the drillhole;
4. Water is added to the drill rod and the rate of decay in the excess water level is measured over time;
5. The test is stopped when the water level decays to static.

The Hvorslev method (1) was used to calculate hydraulic conductivity of the test interval.

Values of hydraulic conductivity determined from falling head tests are generally considered index values indicative of the order of magnitude of hydraulic conductivity. This is normally a sufficient estimate for most practical engineering applications. Better accuracy generally requires extensive aquifer testing techniques using multiple hole pump tests, and are typically not warranted in conditions such as those encountered in the Price River area.

**APPENDIX 7-2**  
**GROUNDWATER SUMMARY SHEETS**

STATION B-001

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	0.08	Dry	0.04	Ammonia	-	-	-
pH	8.1	7.8	7.95	Nitrate	-	-	-
Conductivity	-	-	5150	Nitrite	-	-	-
Temp. C degrees	15	14	14.5	Potassium	-	-	-
(water)							
TSS	34.0	21.0	10.5	Total	-	-	-
				Phosphate			
TDS	4420	1400	2910	Ortho -	-	-	-
				Phosphate			
Hardness	-	-	-	Selenium	-	-	-
Aluminum	-	-	-	Sodium	-	-	-
Arsenic	-	-	-	Sulfate	-	-	-
Barium	-	-	-	Zinc	-	-	-
Boron	-	-	-	Gross Alpha	-	-	-
Carbonate	-	-	-	Gross Beta	-	-	-
Bicarbonate	-	-	-	Oil & Grease	-	-	<1.0
Cadmium	-	-	-	Phenol	-	-	-
Calcium	-	-	-	Alkalinity	668	183	525.5
Chloride	-	-	-	Acidity	100	54.0	77.0
Chromium	-	-	-	Silver	-	-	-
Copper	-	-	-				
Fluoride	-	-	-				

Iron-Total	0.565	0.091	0.292
Iron	-	-	-
Dissolved			
Lead	-	-	-
Magnesium	-	-	-
Manganese	-	-	-
Mercury	-	-	-
Molybdenum	-	-	-
Nickel	-	-	-

STATION BW-23

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	0	0	0	Ammonia	-	-	-
pH	WELL	DRY	-	Nitrate	-	-	-
Conductivity	-	-	-	Nitrite	-	-	-
Temp. C	-	-	-	Potassium	-	-	-
degrees							
(water)							
TSS	-	-	-	Total	-	-	-
				Phosphate	-	-	-
TDS	-	-	-	Ortho-	-	-	-
				Phosphate	-	-	-
Hardness	-	-	-	Selenium	-	-	-
Aluminum	-	-	-	Sodium	-	-	-
Arsenic	-	-	-	Sulfate	-	-	-
Barium	-	-	-	Zinc	-	-	-
Boron	-	-	-	Gross Alpha	-	-	-
Carbonate	-	-	-	Gross Beta	-	-	-
Bicarbonate	-	-	-	Oil & Grease	-	-	-
Cadmium	-	-	-	Phenol	-	-	-
Calcium	-	-	-	Alkalinity	-	-	-
Chloride	-	-	-	Acidity	-	-	-
Chromium	-	-	-	Silver	-	-	-
Copper	-	-	-				
Fluoride	-	-	-				
Iron-Total	-	-	-				
Iron	-	-	-				
Dissolved							
Lead	-	-	-				
Magnesium	-	-	-				
Manganese	-	-	-				
Mercury	-	-	-				
Molybdenum	-	-	-				
Nickel	-	-	-				

STATION EW-24

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	0	0	0	Ammonia	-	-	-
pH	WELL	DRY	-	Nitrate	-	-	-
Conductivity	-	-	-	Nitrite	-	-	-
Temp. C	-	-	-	Potassium	-	-	-
degrees							
(water)							
TSS	-	-	-	Total	-	-	-
				Phosphate	-	-	-
TDS	-	-	-	Ortho-	-	-	-
				Phosphate	-	-	-
Hardness	-	-	-	Selenium	-	-	-
Aluminum	-	-	-	Sodium	-	-	-
Arsenic	-	-	-	Sulfate	-	-	-
Barium	-	-	-	Zinc	-	-	-
Boron	-	-	-	Gross Alpha	-	-	-
Carbonate	-	-	-	Gross Beta	-	-	-
Bicarbonate	-	-	-	Oil & Grease	-	-	-
Cadmium	-	-	-	Phenol	-	-	-
Calcium	-	-	-	Alkalinity	-	-	-
Chloride	-	-	-	Acidity	-	-	-
Chromium	-	-	-	Silver	-	-	-
Copper	-	-	-				
Fluoride	-	-	-				
Iron-Total	-	-	-				
Iron	-	-	-				
Dissolved							
Lead	-	-	-				
Magnesium	-	-	-				
Manganese	-	-	-				
Mercury	-	-	-				
Molybdenum	-	-	-				
Nickel	-	-	-				

STATION B-32

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	0.1	0.08	0.07	Ammonia	-	-	-
pH	8.5	7.9	8.10	Nitrate	0.04	<0.01	0.03
Conductivity	5.90	4.75	5.31	Nitrite	-	-	<0.01
Temp. C	15	8.5	12.13	Potassium	1.43	1.368	1.40
degrees (water)							
TSS	-	-	11.0	Total Phosphate	0.230	0.060	0.150
TDS	365	325	350.50	Ortho- Phosphate	0.050	0.010	0.03
Hardness	-	-	-	Selenium	<0.001	<0.001	<0.001
Aluminum	-	-	-	Sodium	39.20	28.12	33.71
Arsenic	<0.010	<0.001	0.003	Sulfate	72.0	43.0	56.2500
Barium	0.080	0.01	0.06	Zinc	0.016	0.002	0.01
Boron	-	-	-	Gross Alpha	-	-	-
Carbonate	-	-	-	Gross Beta	-	-	-
Bicarbonate	358.68	295.24	324.52	Oil & Grease	2.60	2.00	2.30
Cadmium	<0.001	<0.001	<0.001	Phenol	-	-	0.104
Calcium	45.60	40.00	42.60	Alkalinity	294	250	273.35
Chloride	10.00	28.00	15.28	Acidity	14.0	<0.01	4.70
Chromium	<0.001	<0.001	<0.001	Silver	<0.001	<0.001	<0.001
Copper	0.020	0.001	0.010				
Fluoride	0.36	0.33	0.34				
Iron-Total	0.980	0.030	0.370				
Iron	-	-	-				
Dissolved							
Lead	<0.001	<0.001	<0.001				
Magnesium	47.52	37.92	40.82				
Manganese	0.011	0.003	.01				
Mercury	-	-	0.0002				
Molybdenum	-	-	-				
Nickel	-	-	0.010				

STATION     B-33    

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	0.05	-	0.0158	Ammonia	-	-	-
pH	7.0	7.4	7.6250	Nitrate	1.06	0.03	.3065
Conductivity	490	390	4126	Nitrite	<0.01	<0.01	<0.01
			6667	Potassium	2.25	1.08	1.5058
Temp. C	7.5	4	5.6250	Total Phosphate	0.120	0.020	0.0700
degrees (water)				Ortho-Phosphate	0.024	0.02	0.0220
TSS	68.0	2.0	35.00	Selenium	<0.001	<0.001	<0.001
TDS	380	255	304.00	Sodium	12.93	3.50	8.5325
Hardness	-	-	-	Sulfate	136	43.0	75.500
Aluminum	-	-	-	Zinc	0.049	0.009	0.238
Arsenic	<0.001	<0.001	<0.001	Gross Alpha	-	-	-
Barium	0.16	0.060	0.0875	Gross Beta	-	-	-
Boron	-	-	0.077	Oil & Grease	4.6	<1.0	2.733
Carbonate	-	-	-	Phenol	-	-	<0.001
Bicarbonate	258.64	236.68	243.9850	Alkalinity	212	204	208.00
Cadmium	<0.001	<0.001	<0.001	Acidity	20.	<0.1	10.05
Calcium	63.2	52.00	58.00	Silver	<0.001	<0.001	<0.001
Chloride	14.0	2.7	8.1758				
Chromium	<0.001	<0.001	<0.001				
Copper	0.040	0.004	0.0168				
Fluoride	0.25	0.17	0.2167				
Iron-Total	0.640	0.016	.2143				
Iron	-	-	0.020				
Dissolved Lead	<0.001	<0.001	<0.001				
Magnesium	42.7	24.48	31.1950				
Manganese	0.010	0.001	0.0065				
Mercury	<0.0002	<0.0002	<0.002				
Molybdenum	-	-	-				
Nickel	-	-	<0.01				

STATION B-41

MC 205?

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	-	-	-	Ammonia	-	-	-
pH	-	-	8.4	Nitrate	-	-	-
Conductivity	-	-	1700	Nitrite	-	-	-
Temp. C	-	-	12	Potassium	-	-	14.40
degrees (water)							
TSS	-	-	-	Total	-	-	-
				Phosphate	-	-	-
TDS	-	-	-	Ortho-	-	-	-
				Phosphate	-	-	-
Hardness	-	-	-	Selenium	-	-	-
Aluminum	-	-	-	Sodium	-	-	90.20
Arsenic	-	-	-	Sulfate	-	-	240
Barium	-	-	-	Zinc	-	-	-
Boron	-	-	-	Gross Alpha	-	-	-
Carbonate	-	-	-	Gross Beta	-	-	-
Bicarbonate	-	-	378.20	Oil & Grease	-	-	-
Cadmium	-	-	-	Phenol	-	-	-
Calcium	-	-	155.20	Alkalinity	-	-	310
Chloride	-	-	380	Acidity	-	-	3.20
Chromium	-	-	-	Silver	-	-	-
Copper	-	-	-				
Fluoride	-	-	-				
Iron-Total	-	-	0.182				
Iron	-	-	0.040				
Dissolved							
Lead	-	-	-				
Magnesium	-	-	121.44				
Manganese	-	-	0.068				
Mercury	-	-	-				
Molybdenum	-	-	-				
Nickel	-	-	-				

STATION B-42 MC 206

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	-	-	-	Ammonia	-	-	-
pH	8.0	7.7	7.80	Nitrate	0.04	0.01	0.03
Conductivity	2550	1500	201	Nitrite	-	-	-
Temp. C	20	10	15.125	Potassium	7.37	5.00	6.2425
degrees (water)				Total	0.550	0.080	0.32
TSS	-	-	-	Phosphate			
TDS	1887	1350	1614.	Ortho-	-	-	-
Hardness	-	-	-	Phosphate			
Aluminum	-	-	-	Selenium	0.001	0.001	0.001
Arsenic	1.0620	<0.001	0.5407	Sodium	588	82.10	381.2750
Barium	0.140	0.045	0.09	Sulfate	1160	150	528.500
Boron	-	-	0.150	Zinc	0.300	0.030	0.17
Carbonate	-	-	-	Gross Alpha	-	-	-
Bicarbonate	1246.84	195.20	792.	Gross Beta	-	-	-
Cadmium	0.001	<0.001	<0.001	Oil & Grease	252.2	22.40	137.30
Calcium	196.6	20.0	77.950	Phenol	0.053	0.004	0.03
Chloride	126	28.0	84.400	Alkalinity	1022	160	657.500
Chromium	0.017	<0.001	0.01	Acidity	42.0	<0.01	12.5050
Copper	0.45	0.020	0.03	Silver	0.005	0.001	3.0
Fluoride	8.07	2.92	5.50				
Iron-Total	264	0.136	66.4				
Iron	23.60	0.098	11.8				
Dissolved			490				
Lead	<0.001	<0.001	<0.001				
Magnesium	80.64	15.36	47.7				
Manganese	0.504	0.015	0.1948				
Mercury	-	-	0.0002				
Molybdenum	-	-	-				
Nickel	-	-	0.016				

STATION B-43 MC 207

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	-	-	-	Ammonia	-	-	-
pH	7.783	7.78	7.6125	Nitrate	0.54	0.02	0.3433
Conductivity	1820	1350	167	Nitrite	-	-	-
			.500	Potassium	11.00	4.50	7.7150
Temp. C degrees (water)	14	10	11.50	Total Phosphate	0.800	0.380	.5667
TSS	-	-	363	Ortho- Phosphate	-	-	-
TDS	1679	1120	1024. 75	Selenium	<0.001	<0.001	<0.001
Hardness	-	-	-	Sodium	350	310	333.750
Aluminum	-	-	-	Sulfate	619	48.0	258.500
Arsenic	0.240	<0.001	0.0807	Zinc	0.220	0.020	0.0917
Barium	0.100	0.070	0.0820	Gross Alpha	-	-	-
Boron	-	-	0.130	Gross Beta	-	-	-
Carbonate	-	-	-	Oil & Grease	25000	107.20	12553.60
Bicarbonate	1122.40	795.44	979. 0500	Phenol	0.048	0.015	0.031
Cadmium	0.003	0.001	0.0017	Alkalinity	920	652	802.50
Calcium	226.4	48.00	104.200 200	Acidity	148	6.0	49.3500
Chloride	66.00	36.8	54.700	Silver	<0.001	<0.001	<0.001
Chromium	0.007	<0.001	0.0030				
Copper	.080	.014	.0513				
Fluoride	1.53	1.50	1.52				
Iron-Total	4.550	0.120	1.42 25				
Iron	0.124	0.080	0.10 20				
Dissolved							
Lead	.073	.073	.073				
Magnesium	52.80	16.32	37.6 200				
Manganese	.140	0.012	0.0930				
Mercury	-	-	<0.0 002				
Molybdenum	-	-	-				
Nickel	-	-	0.022				

STATION BM-25

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	-	-	-	Ammonia	8.0	0.15	1.4400
pH	8.16	6.9	7.4786	Nitrate	9.00	0.02	1.4786
Conductivity	2050	1390	1646	Nitrite	1.05	1.02	1.0350
Temp. C	13	12	12.50	Potassium	17.80	6.52	10.334
degrees (water)				Total	2.80	0.090	0.6451
TSS	1900	32.0	354. 4286	Phosphate	0.54	0.020	0.2051
TDS	1350	906	1080. 5741	Ortho- Phosphate	<0.001	<0.001	<0.001
Hardness	600	530	565. 4286	Selenium	185	6.50	137.1429
Aluminum	0.036	<0.001	0.0185	Sodium	580	400	362
Arsenic	<0.001	<0.001	<0.001	Sulfate	0.408	0.011	0.0913
Barium	0.110	0.005	0.0534	Zinc	-	-	-
Boron	0.30	<0.001	0.1494	Gross Alpha	-	-	-
Carbonate	19.20	<0.001	2.7514	Gross Beta	-	-	-
Bicarbonate	558.76	268.4	427. 5657	Oil & Grease	4070	3.4	599.2571
Cadmium	<0.001	<0.001	<0.001	Phenol	<0.001	<0.001	<0.001
Calcium	123.2	80.0	7.00	Alkalinity	490	2.54	366.2857
Chloride	150	46.0	99.1 429	Acidity			
Chromium	0.013	<0.001	0.0027	Silver	0.051	<0.001	0.0081
Copper	.005	.004	.0043				
Fluoride	0.66	0.50	0.5743				
Iron-Total	3.42	0.072	1.07 57				
Iron Dissolved							
Lead	.004	<0.001	0.0017				
Magnesium	79.20	60.94	71.5 857				
Manganese	0.218	0.023	0.0734				
Mercury	<0.0002	<0.0002	<0.0 002				
Molybdenum	0.029	<0.001	0.0150				
Nickel	0.169	<0.001	0.0850				

STATION BM-26

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	0.01	0.01	0.01	Ammonia	8.00	0.10	7.38
pH	8.2	7.33	7.78	Nitrate	0.84	0.05	0.14
Conductivity	1990	1050	1312	Nitrite	<0.01	<0.01	<0.01
			.83	Potassium	15.51	6.00	6.65
Temp. C degrees (water)	9	8	8.25	Total Phosphate	2.30	0.150	0.91
TSS	1228	100	633.	Ortho- Phosphate	0.150	0.04	0.09
TDS	1300	700	861.	Selenium	0.005	<0.001	0.003
Hardness	522	470	498.	Sodium	65	18.5	42.64
Aluminum	<0.001	<0.001	<0.001	Sulfate	640	360	430
Arsenic	<0.001	<0.001	<0.001	Zinc	0.408	0.043	135.72
Barium	0.055	0.012	0.03	Gross Alpha	-	-	-
Boron	0.30	<0.001	0.18	Gross Beta	-	-	-
Carbonate	<0.001	<0.001	<0.001	Oil & Grease	13.6	1.7	6.90
Bicarbonate	270.8	204.96	228.	Phenol	<0.001	<0.001	<0.001
			12	Alkalinity	277	168	187
Cadmium	<0.001	<0.001	<0.001	Acidity			
Calcium	112	88	100.67	Silver	<0.001	<0.001	<0.001
Chloride	32.0	24.0	24.33				
Chromium	<0.001	<0.001	<0.001				
Copper	0.005	<0.001	0.003				
Fluoride	0.51	0.36	0.43				
Iron-Total	16.430	0.051	6.00				
Iron Dissolved							
Lead	0.003	<0.001	0.001				
Magnesium	62.4	55.2	59.28				
Manganese	0.227	0.021	0.13				
Mercury	<0.0002	<0.0002	<0.0				
			002				
Molybdenum	0.002	0.002	0.002				
Nickel	<0.001	<0.001	<0.001				

STATION BM-27

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	0.002	0	0.001	Ammonia	0.36	<0.01	0.2300
pH	8.0	7.63	7.800	Nitrate	0.70	0.06	0.300
Conductivity	1790	1200	1509.500	Nitrite	<0.01	<0.01	<0.01
Temp. C degrees (water)	9	9	9.0	Potassium	9.11	5.82	7.7925
TSS	14.0	4.0	10.250	Total Phosphate	2.30	0.060	0.6638
TDS	1200	964	1082.500	Ortho-Phosphate	0.040	0.015	0.0238
Hardness	810	740	762.5000	Selenium	<0.001	<0.001	<0.001
Aluminum				Sodium	102	23.0	51.475
Arsenic	<0.001	<0.001	<0.001	Sulfate	660	560	590.00
Barium	0.070	0.050	0.0550	Zinc	0.082	0.020	0.0563
Boron	0.250	0.13	0.1900	Gross Alpha	-	-	-
Carbonate	<0.01	<0.01	<0.01	Gross Beta	-	-	-
Bicarbonate	436.7	200	267.4000	Oil & Grease	5.2	<1.0	2.7500
Cadmium	<0.001	<0.001	<0.001	Phenol			
Calcium	170.4	152	162.1384	Alkalinity	358	164	219.00
Chloride	34.0	28.0	31.00	Acidity			
Chromium	<0.001	<0.001	<0.001	Silver	<0.001	<0.001	<0.001
Copper							
Fluoride	0.50	0.38	0.4350				
Iron-Total	0.409	0.149	0.3198				
Iron Dissolved							
Lead	0.005	<0.001	0.0030				
Magnesium	92.16	84.0	87.2400				
Manganese	0.156	0.004	0.0503				
Mercury	<0.0002	<0.0002	<0.0002				
Molybdenum							
Nickel							

STATION BM-28

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	-	-	0.005	Ammonia	0.35	0.05	0.154
pH	-7.9	7.2	7.6080	Nitrate	0.06	0.03	0.224
Conductivity	1850	1380	1538.600	Nitrite	-	-	<0.01
Temp. C degrees (water)	7	4	5.7500	Potassium	7.03	4.76	6.1100
TSS	31.0	7.0	14.6000	Total Phosphate	0.740	0.04	0.1798
TDS	1240	898	1007.2000	Ortho-Phosphate	0.043	<0.01	0.0184
Hardness	830	660	776.00	Selenium	0.004	<0.001	0.0016
Aluminum	-	-	-	Sodium	56.0	15.0	28.000
Arsenic	0.014	<0.001	0.0036	Sulfate	460	360	404.00
Barium	0.057	0.025	0.0404	Zinc	0.074	0.012	0.0368
Boron	0.20	0.075	0.1400	Gross Alpha	-	-	-
Carbonate	<0.01	<0.01	<0.01	Gross Beta	-	-	-
Bicarbonate	488	456.28	467.4960	Oil & Grease	7.4	<1.0	3.1800
Cadmium	<0.001	<0.001	<0.001	Phenol	-	-	-
Calcium	316	180	207.0400	Alkalinity	400	374	383.200
Chloride	32.0	20.0	26.800	Acidity	-	-	-
Chromium	<0.001	<0.001	<0.001	Silver	<0.001	<0.001	<0.001
Copper	-	-	0.004				
Fluoride	0.44	0.33	0.3840				
Iron-Total	4.390	0.438	2.9244				
Iron Dissolved	-	-	-				
Lead	0.004	<0.001	0.0018				
Magnesium	96.48	<0.01	62.0180				
Manganese	0.395	0.039	0.2798				
Mercury	<0.0002	<0.0002	<0.0002				
Molybdenum	-	-	-				
Nickel	-	-	-				

**APPENDIX 7-3**  
**ALLUVIAL VALLEY FLOORS**

- ALLUVIAL VALLEY FLOORS

Alluvial valley floors considerations were not originally thoroughly addressed in the M.R.P. due to company interpretation of the applicable regulations. The following points are considered significant by Price River Coal Company as related to AVF determinations:

1. There are no farmable land units on PRCC property.
2. There has been a negative S.C.S. PFL determination.
3. Their closest serious farming in the Price River valley is in Spring Glen, some 8 miles away from the MPA.
4. Minor hobby and backyard farming occurs within 2 miles of the MPA, in the town of Martin, but is being encroached upon by urban development.
5. Mining and farming have co-existed in the Price River drainage since 1890 with no significant or measured diminution of quantity or quality (statement from Area Water Engineer, M. Page).
6. PRCC and its predecessors have monitored quantity and quality of the Price River since 1977 with no noticeable effects from mining.
7. Several significant surface disturbances by non-mining activities exist between PRCC facilities and potential AVFs, i.e., UP&L power plant, UP&L coal storage piles, treatment ponds and flyash disposal site, a UDOT coal, ash and salt storage site, the DRGW train repair yard. None of these sites benefit from drainage protection for the Price River.

The foregoing facts caused confusion as to the applicability of AVF regulations. In June of 1982 PRCC requested aid from the R.A. in evaluating our situation. The following comments were provided by DOGM as result of a joint field survey on August 9, 1982.



STATE OF UTAH  
 NATURAL RESOURCES & ENERGY  
 Oil, Gas & Mining

James W. Smith, Jr.  
 Director  
 Office of Natural Resources & Energy  
 150 East 100 South, Salt Lake City, Utah 84143

2025 State of Utah - Salt Lake City, Utah 84143-3000

August 25, 1982

Mr. Rob Wiley  
 Price River Coal Company  
 P.O. Box 629  
 Helper, Utah 84526

RE: AVE DETERMINATION  
 PRICE RIVER COMPLEX  
 ACT/007/004  
 CARBON COUNTY, UTAH

Dear Rob:

Enclosed is the Division determination of the existence and status of alluvial valley floors on and adjacent to Price River Coal Company's mine plan. These determinations were made as a result of the investigation conducted on August 9, 1982.

If you have any questions or comments, please contact me, Tom Tetting or Dave Darby.

Sincerely,

JAMES W. SMITH, JR.  
 COORDINATOR OF MINED  
 LAND DEVELOPMENT

cc. Allen Klein, OSM, DENVER  
 David Darby, DOOM  
 Tom Tetting, DOOM

JWS/DD/

ALLUVIAL VALLEY FLOOR DETERMINATION  
OF PRICE RIVER COAL COMPANY'S  
MINE PLAN AND ADJACENT AREAS

Introduction

In response to OSM's comments pertaining to alluvial valley floors in the Apparent Completeness review dated May 29, 1982, Price River Coal Company requested that a field determination be made of their project and adjacent areas by a regulatory agency to evaluate the existence of any alluvial valley floor.

On August 9, 1982, personnel from the Division of Oil, Gas and mining representing various disciplines toured Price River Coal Company's mine permit area and areas adjacent to the mining project specifically to determine the existence of any alluvial valley floors and if existent, to what extent mining could affect the alluvial valley floors.

Federal and State regulations provide for the protection of alluvial valley floors from mining activities. No coal mining operation can materially damage the quantity or quality of surface or groundwater systems which supply alluvial valley floors, and mining operations may not interrupt, discontinue, or preclude forming an alluvial valley floor. By definition alluvial valley floors are:

" . . .the unconsolidated stream laid deposits holding streams with water available sufficient for subirrigation or flood irrigation agricultural activities but does not include unplanned areas which are generally overlain by thin veneer of colluvial deposits formed by unconcentrated runoff or slope wash, together

with talus, or other mass-movement accumulations, and windblown deposits"

In a general sense, alluvial valley floors are those areas which are located in topographic valleys, which are underlain by any unconsolidated deposits which usually have a landform appearance of floodplains or terraces, which have an agricultural importance derived from the availability of surface or groundwater.

Applicants are required to make initial identifications of alluvial valley floors based on readily or easily obtainable data, and conduct detailed studies only on specific problem areas.

#### Findings

Tom Tetting, Engineering Geologist; Everett Hooper, Soils Specialist; Lynn Kunzler, Biologist; and David Darby, Hydrologist; from the Division of Oil, Gas and Mining met with Rob Wiley, Environmental Engineer; and Don Stephens, Geologist; representatives of Price River Coal Company.

Preliminary discussions took place at Price River Coal Company's office in Helper, Utah. Maps were examined which depicted historic, present and future mining areas as well as topographic and geomorphic features. Potential AVF sites along Willow Creek, the Price River, Spring Canyon and Kenilworth, Utah were chosen for investigation.

The conditions along Willow Creek and the Price River within the mine plan area are limited with respect to the existence of AVFs. State highways, railroad tracks, precipitous slopes, river width and narrow canyon walls account for only a few areas that qualify as AVF

under the specified size criteria of an area 50 feet wide or 10 acres in size. Such areas have been utilized already for a sewage treatment facility, an electrical power plant and the coal loadout for Price River Coal Company which were there prior to the passage of SMCRA. Several areas up Willow Creek have been used as coal waste dumps during earlier mining times.

Historically there has not been any farming along the Price River or Willow Creek. Price River Coal Company presently owns all surface rights along the rivers and during the process of mining foresees no change in that status.

Outside the mine plan area three sites were investigated for their potential as an AVF. The area surrounding the town of Martin, two miles downstream from the eastern edge of the mine plan area along the Price River, showed good conditions for an AVF. The area lies on a pediment formed by stream laden deposits of the Price River and include several small parcels of land developed (approximately 41 acres) for agriculture (corn and alfalfa and one orchard) by several individuals. Upon further examination, it was learned that irrigation waters for farming are supplied via canal diverted from the Price River. No springs or wells occur near the town of Martin for irrigation or culinary purposes.

Spring Canyon was another area examined where a small orchard (approximately 2 acres) exists. Communications with the owner revealed that the orchard has been abandoned for 11 or 12 years. Water from two springs about 150 yards an about 1/4 miles above the orchard was used

for irrigation. The flow rate at the present time is approximately 35 to 40 gpm. The owner stated that the flow had decreased in the last 20 years although no records or data of flow exist. The most significant decrease took place after several mines closed down.

The area surrounding the town of Kenilworth was investigated due to its location adjacent to the mine plan area. The investigation revealed that it is located on a pediment of the Manchos shale. although flora exists amidst the residential section, there is no farming. Water is supplied through a public water system. The stream channels are ephemeral in nature and do not contribute to irrigation.

#### Determination

Alluvial valley floor investigations within and adjacent to the proposed Price River Coal Company's permit area has resulted in the determination that the criteria necessary to establish an alluvial valley floor does exist to a small extent within the river channels of the permit area, and to a great extent near the suburbs of Martin immediately south of the mine plan area and in Spring Canyon.

In view of information presented during the tour it was determined that:

1. The Price River is monitored above and below the coal processing plant to access changes in water quality.
2. Historic mining in 30 to 40 mines has existed in and adjacent to the mine plan area along the escarpment which lies above Kenilworth, Spring Canyon, Martin and Helper. Also, the Price River has been mined under several times. It is

speculated that any groundwater that could supply those areas has already been affected. Since present mining occurs farther down dip and away from the AVF there is less chance that mining will have any effect on the AVFs.

3. No farming or agricultural activity takes place on the mine plan area, and therefore the small areas along the river channels that could be classified by definition as AVFs are insignificant due to their isolation and size.
4. There was no finding near the town of Kenilworth that indicated an AVF exists.
5. There are no wells used in and adjacent to the mine plan area. The springs up spring Canyon are used by one family to water their lawn. All other irrigation uses water is supplied via canal by the Price River which in turn is regulated from Scofield Reservoir. All culinary water is supplied from springs near Scofield Reservoir and from the water treatment plant north of the coal processing facility which takes water from the Price River.

The Division has made the determination that present and future mining will not change the status or condition of the water resources, soils or geology relating to alluvial valley floors in or adjacent to the mine plan area. Mining will not interrupt or cause diminution of the existing ground water or irrigation waters in a significant manner.

































VAUGHN HANSEN ASSOCIATES  
 Consultants / Engineers

WATER QUALITY DATA

Station MC-205 (B-41)  
 Location Sawbrilly Drill Hole

DATE	TIME	TEMPERATURE			SOLIDS			OIL & GREASE			PHOSPHORUS			NITROGEN			AMMONIA			SILICA			COND.	TURBIDITY	DO	pH	TOTAL ALKALINITY	TOTAL HARDNESS	CALC. HARDNESS	CHLORIDE	SULFATE	CATIONS	ANIONS		
		AIR	WATER	SOIL	TOTAL	FEED	STOCK	WATER	WATER	WATER	TOTAL	WATER	WATER	TOTAL	WATER	WATER	TOTAL	WATER	WATER	TOTAL	WATER	WATER												TOTAL	
6-23-80	13:15				7.1																														
6-23-80	14:56																																		
6-23-80	16:20																																		
6-23-80	17:50																																		
6-23-80	18:30		18	10	6.7																														
6-23-80	19:40		16	9	7.8					1.4	0.05	0.01			0.070	538																			
6-23-80	19:50		19	11	7.8					0.2	0.136	0.05			0.15	448																			

DATE	TIME	SILICA		AMMONIA		NITROGEN		PHOSPHORUS		SULFATE		CALC. HARDNESS		TOTAL HARDNESS		CATIONS		ANIONS		COND.	TURBIDITY	DO	pH	TOTAL ALKALINITY	TOTAL HARDNESS	CALC. HARDNESS	CHLORIDE	SULFATE	CATIONS	ANIONS							
		TOTAL	WATER	TOTAL	WATER	TOTAL	WATER	TOTAL	WATER	TOTAL	WATER	TOTAL	WATER	TOTAL	WATER	TOTAL	WATER	TOTAL	WATER												TOTAL						
6-23-80	13:15																																				
6-23-80	14:56																																				
6-23-80	16:20																																				
6-23-80	17:50																																				
6-23-80	18:30																																				
6-23-80	19:40		0.003	0.100																																	
6-23-80	19:50		0.001	0.096																																	
6-23-80	19:55		0.001	0.100																																	

MC-205A-1st Turned Stop  
 Not enough water to Sample  
 Not enough water to Sample  
 Not enough water to Sample



























**APPENDIX 7-5**  
**SURFACE WATER SUMMARY SHEETS**

STATION B-26 Crandall Canyon Below Site

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	0	0	0	Ammonia	-	-	-
pH	-	-	-	Nitrate	-	-	-
Conductivity	-	-	-	Nitrite	-	-	-
Temp. C	-	-	-	Potassium	-	-	-
degrees							
(water)							
TSS	-	-	-	Total	-	-	-
				Phosphate	-	-	-
TDS	-	-	-	Ortho-	-	-	-
				Phosphate	-	-	-
Hardness	-	-	-	Selenium	-	-	-
Aluminum	-	-	-	Sodium	-	-	-
Arsenic	-	-	-	Sulfate	-	-	-
Barium	-	-	-	Zinc	-	-	-
Boron	-	-	-	Gross Alpha	-	-	-
Carbonate	-	-	-	Gross Beta	-	-	-
Bicarbonate	-	-	-	Oil & Grease	-	-	-
Cadmium	-	-	-	Phenol	-	-	-
Calcium	-	-	-	Alkalinity	-	-	-
Chloride	-	-	-	Acidity	-	-	-
Chromium	-	-	-	Silver	-	-	-
Copper	-	-	-				
Fluoride	-	-	-				
Iron-Total	-	-	-				
Iron	-	-	-				
Dissolved							
Lead	-	-	-				
Magnesium	-	-	-				
Manganese	-	-	-				
Mercury	-	-	-				
Molybdenum	-	-	-				
Nickel	-	-	-				

STATION B-25 Crandall Canyon Above Site #3

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	0	0	0	Ammonia	-	-	-
pH	-	-	-	Nitrate	-	-	-
Conductivity	-	-	-	Nitrite	-	-	-
Temp. C	-	-	-	Potassium	-	-	-
degrees							
(water)							
TSS	-	-	-	Total	-	-	-
				Phosphate			
TDS	-	-	-	Ortho-	-	-	-
				Phosphate			
Hardness	-	-	-	Selenium	-	-	-
Aluminum	-	-	-	Sodium	-	-	-
Arsenic	-	-	-	Sulfate	-	-	-
Barium	-	-	-	Zinc	-	-	-
Boron	-	-	-	Gross Alpha	-	-	-
Carbonate	-	-	-	Gross Beta	-	-	-
Bicarbonate	-	-	-	Oil & Grease	-	-	-
Cadmium	-	-	-	Phenol	-	-	-
Calcium	-	-	-	Alkalinity	-	-	-
Chloride	-	-	-	Acidity	-	-	-
Chromium	-	-	-	Silver	-	-	-
Copper	-	-	-				
Fluoride	-	-	-				
Iron-Total	-	-	-				
Iron	-	-	-				
Dissolved							
Lead	-	-	-				
Magnesium	-	-	-				
Manganese	-	-	-				
Mercury	-	-	-				
Molybdenum	-	-	-				
Nickel	-	-	-				

STATION B-21 Ford Creek Above U.S. 50 & 6

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	11.6	-	1.28	Ammonia	-	-	-
pH	9.1	7.51	8.16	Nitrate	0.11	<0.01	0.03
Conductivity	580	250	475	Nitrite	-	-	<0.01
Temp. C	16.5	-	8.2	Potassium	1.98	0.96	1.32
degrees (water)							
TSS	169	1.0	43.3	Total	0.34	<0.001	0.16
				Phosphate			
TDS	370	252	319	Ortho-	0.49	<0.01	0.12
				Phosphate			
Hardness	-	-	1180	Selenium	0.002	<0.001	0.001
Aluminum	-	-	0.037	Sodium	19.0	48.3	9.56
Arsenic	0.015	<0.001	0.001	Sulfate	73.0	4.0	49.32
Barium	0.270	0.050	0.110	Zinc	0.03	<0.001	0.01
Boron	0.05	0.03	0.04	Gross Alpha	-	-	-
Carbonate	-	-	<0.01	Gross Beta	-	-	-
Bicarbonate	390.4	256.2	296.8	Oil & Grease	18.8	<1.0	3.03
Cadmium	<0.001	<0.001	<0.001	Phenol	0.035	<0.001	0.00
Calcium	96.8	64.0	57.4	Alkalinity	320	210	247
Chloride	14.0	4.0	8.6	Acidity	44.0	<0.01	12.01
Chromium	0.002	<0.001	<0.001	Silver	0.001	<0.001	<0.001
Copper	0.02	<0.001	0.006				
Fluoride	0.24	0.15	0.19				
Iron-Total	6.26	0.035	0.86				
Iron	-	-	0.01				
Dissolved							
Lead	0.004	<0.001	0.0021				
Magnesium	47.40	12.00	35.12				
Manganese	0.09	0.002	0.02				
Mercury	<0.0002	<0.0002	<0.0002				
			002				
Molybdenum	-	-	-				
Nickel	-	-	-				

STATION B-20 Price River above Sulfur Canyon Creek

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	425	20.0	167	Ammonia	-	-	-
pH	9.5	7.0	8.1	Nitrate	0.4	<0.01	0.12
Conductivity	660	280	424	Nitrite	-	-	-
Temp. C degrees (water)	27	-	8.2	Potassium	1.902	0.85	1.40
TSS	1140	1.0	125.5	Total Phosphate	0.35	0.005	0.16
TDS	432	198	303	Ortho- Phosphate	0.04	<0.01	0.01
Hardness	-	-	-	Selenium	0.004	<0.001	0.001
Aluminum	-	-	-	Sodium	88.0	5.92	23.87
Arsenic	0.020	<0.001	0.001	Sulfate	148	5.0	60.6
Barium	0.245	0.060	0.114	Zinc	0.062	<0.001	0.014
Boron	0.07	0.03	0.05	Gross Alpha	-	-	-
Carbonate	-	-	<0.01	Gross Beta	-	-	-
Bicarbonate	366	148.84	256.67	Oil & Grease	57.0	<1.0	5.24
Cadmium	<0.001	<0.001	<0.001	Phenol	0.281	<0.001	0.030
Calcium	88	45.6	60	Alkalinity	266	122	209
Chloride	34	2.0	12.2	Acidity	32.0	<0.01	9.63
Chromium	<0.001	<0.001	<0.001	Silver	<0.001	<0.001	<0.001
Copper	0.030	<0.001	0.006				
Fluoride	0.36	0.001	0.18				
Iron-Total	24.50	0.053	2.35				
Iron	0.030	0.020	0.025				
Dissolved							
Lead	0.011	<0.001	0.001				
Magnesium	37.92	4.80	22.21				
Manganese	0.339	<0.001	0.072				
Mercury	<0.0002	<0.0002	<0.0002				
Molybdenum	-	-	-				
Nickel	-	-	-				

STATION B-19 Sulfur Canyon Creek

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	7.0	0.004	1.11	Ammonia	-	-	-
pH	9.2	7.0	8.1	Nitrate	0.34	<0.02	0.04
Conductivity	930	325	663	Nitrite	0.04	<0.01	0.02
Temp. C degrees (water)	21	0	7.1	Potassium	12.5	1.43	1.897
TSS	215	<1.0	35.6	Total Phosphate	0.560	<0.001	0.123
TDS	639	315	455.7	Ortho- Phosphate	0.300	<0.01	0.065
Hardness	-	-	-	Selenium	0.004	<0.001	0.001
Aluminum	-	-	-	Sodium	92.00	19.38	46.70
Arsenic	0.025	<0.001	0.001	Sulfate	166.0	40.0	90.2
Barium	0.25	0.073	0.17	Zinc	0.227	<0.001	0.03
Boron	0.10	0.07	0.09	Gross Alpha	-	-	-
Carbonate	-	-	<0.01	Gross Beta	-	-	-
Bicarbonate	453.84	197.64	349.35	Oil & Grease	24.20	<0.01	3.22
Cadmium	<0.001	<0.001	<0.001	Phenol	0.222	<0.001	0.04
Calcium	120	43.2	68.0	Alkalinity	384	162	266
Chloride	54.3	10.0	28.0	Acidity	38.0	<0.01	11.8
Chromium	0.003	<0.001	0.001	Silver	0.002	<0.001	0.001
Copper	0.040	<0.001	0.008				
Fluoride	0.56	0.21	0.31				
Iron-Total	6.14	0.04	0.80				
Iron	0.05	0.04	0.045				
Dissolved							
Lead	0.003	<0.001	0.001				
Magnesium	53.76	5.28	37.89				
Manganese	0.150	0.004	0.035				
Mercury	<0.0002	<0.0002	<0.0 002				
Molybdenum	-	-	-				
Nickel	-	-	-				

STATION B-17 Sowbelly Creek above Spring Canyon Creek

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	0.34	0	0.03	Ammonia	-	-	0.28
pH	9.1	6.9	7.6	Nitrate	6.2	0.02	3.02
Conductivity	2800	1350	2164	Nitrite	<0.01	<0.01	<0.01
Temp. C	17	5.5	13	Potassium	13.90	8.93	12.5
degrees (water)							
TSS	22.0	<1.0	7.3	Total Phosphate	0.09	0.04	0.06
TDS	1835	967	1489	Ortho- Phosphate	0.05	<0.01	0.02
Hardness	-	-	1180	Selenium	0.006	<0.001	0.003
Aluminum	-	-	0.037	Sodium	188.0	30.0	80.2
Arsenic	-	-	<0.001	Sulfate	1060	460	809
Barium	0.081	0.03	0.05	Zinc	0.089	0.014	0.03
Boron	-	-	0.425	Gross Alpha	-	-	-
Carbonate	-	-	<0.01	Gross Beta	-	-	-
Bicarbonate	514.84	295.24	442.84	Oil & Grease	4.8	<1.0	1.6
Cadmium	<0.001	<0.001	<0.001	Phenol	0.028	<0.001	<0.001
Calcium	430.4	64.0	288.7	Alkalinity	422	242	374.0
Chloride	58.0	4.0	33.3	Acidity	82.0	20.0	48.0
Chromium	<0.001	<0.001	<0.001	Silver	0.002	<0.001	0.001
Copper	0.009	0.004	0.007				
Fluoride	0.47	0.24	0.38				
Iron-Total	0.188	<0.01	0.075				
Iron	-	-	-				
Dissolved							
Lead	0.007	<0.001	0.001				
Magnesium	172.8	46.56	1219				
Manganese	0.016	0.009	0.012				
Mercury	<0.0002	<0.0002	<0.0002				
Molybdenum	-	-	-				
Nickel	-	-	-				

STATION B-14 Holding Pond Below Mine #3

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	0	0	0	Ammonia	-	-	-
pH	-	-	-	Nitrate	-	-	-
Conductivity	-	-	-	Nitrite	-	-	-
Temp. C	-	-	-	Potassium	-	-	-
degrees (water)							
TSS	-	-	-	Total	-	-	-
				Phosphate	-	-	-
TDS	-	-	-	Ortho-	-	-	-
				Phosphate	-	-	-
Hardness	-	-	-	Selenium	-	-	-
Aluminum	-	-	-	Sodium	-	-	-
Arsenic	-	-	-	Sulfate	-	-	-
Barium	-	-	-	Zinc	-	-	-
Boron	-	-	-	Gross Alpha	-	-	-
Carbonate	-	-	-	Gross Beta	-	-	-
Bicarbonate	-	-	-	Oil & Grease	-	-	-
Cadmium	-	-	-	Phenol	-	-	-
Calcium	-	-	-	Alkalinity	-	-	-
Chloride	-	-	-	Acidity	-	-	-
Chromium	-	-	-	Silver	-	-	-
Copper	-	-	-				
Fluoride	-	-	-				
Iron-Total	-	-	-				
Iron	-	-	-				
Dissolved							
Lead	-	-	-				
Magnesium	-	-	-				
Manganese	-	-	-				
Mercury	-	-	-				
Molybdenum	-	-	-				
Nickel	-	-	-				

STATION B-12 Hardscrabble Creek Above Mine #3

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	0	0	0	Ammonia	-	-	-
pH	-	-	-	Nitrate	-	-	-
Conductivity	-	-	-	Nitrite	-	-	-
Temp. C	-	-	-	Potassium	-	-	-
degrees							
(water)							
TSS	-	-	-	Total	-	-	-
				Phosphate	-	-	-
TDS	-	-	-	Ortho-	-	-	-
				Phosphate	-	-	-
Hardness	-	-	-	Selenium	-	-	-
Aluminum	-	-	-	Sodium	-	-	-
Arsenic	-	-	-	Sulfate	-	-	-
Barium	-	-	-	Zinc	-	-	-
Boron	-	-	-	Gross Alpha	-	-	-
Carbonate	-	-	-	Gross Beta	-	-	-
Bicarbonate	-	-	-	Oil & Grease	-	-	-
Cadmium	-	-	-	Phenol	-	-	-
Calcium	-	-	-	Alkalinity	-	-	-
Chloride	-	-	-	Acidity	-	-	-
Chromium	-	-	-	Silver	-	-	-
Copper	-	-	-				
Fluoride	-	-	-				
Iron-Total	-	-	-				
Iron	-	-	-				
Dissolved							
Lead	-	-	-				
Magnesium	-	-	-				
Manganese	-	-	-				
Mercury	-	-	-				
Molybdenum	-	-	-				
Nickel	-	-	-				

STATION B-11 Hardscrabble Creek Below Mine #3

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	0.190	-	0.017	Ammonia	10.25	0.14	5.59
pH	8.60	7.04	7.54	Nitrate	1.40	<0.01	0.27
Conductivity	1900	524	941	Nitrite	-	-	-
Temp. C degrees (water)	25	1	16.4	Potassium	17.9	3.95	8.76
TSS	22596	13	2.598	Total Phosphate	1.10	0.033	0.46
TDS	1260	339	629	Ortho- Phosphate	2.40	0.010	0.001
Hardness	380	104	200	Selenium	0.022	<0.001	0.001
Aluminum	0.15	0.02	0.07	Sodium	330	9.5	108.1
Arsenic	0.004	<0.001	0.001	Sulfate	430	4.0	120.4
Barium	0.37	0.01	0.14	Zinc	0.206	0.015	0.074
Boron	0.860	0.070	0.287	Gross Alpha	-	-	-
Carbonate	19.2	0.070	0.287	Gross Beta	-	-	-
Bicarbonate	705.2	136.6	400.2	Oil & Grease	35.4	1.6	12.7
Cadmium	<0.001	<0.001	<0.001	Phenol	0.143	<0.001	0.03
Calcium	104	20.8	50.9	Alkalinity	578	112	346
Chloride	134	12	45.25	Acidity	56	12	40.5
Chromium	0.007	<0.001	0.001	Silver	0.001	<0.001	<0.001
Copper	0.045	0.006	0.001				
Fluoride	0.045	0.006	0.016				
Iron-Total	117	0.002	12.15				
Iron	0.92	0.92	0.92				
Dissolved							
Lead	0.011	<0.001	0.003				
Magnesium	60.0	6.24	28.93				
Manganese	0.788	0.018	0.202				
Mercury	<0.0002	<0.0002	<0.0002				
Molybdenum	-	-	-				
Nickel	-	-	-				

STATION B-9 Holding Pond Below Mine #2

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	-	-	-	Ammonia	0.95	0.30	0.57
pH	8.7	7.0	7.6	Nitrate	0.16	0.02	0.09
Conductivity	3300	1250	2494	Nitrite	-	-	-
Temp. C	19	4	13.6	Potassium	17.20	2.55	9.60
degrees (water)							
TSS	440	18	98.3	Total Phosphate	0.540	0.015	0.185
TDS	2125	810	1568	Ortho- Phosphate	0.100	0.010	0.049
Hardness	820	44	347	Selenium	<0.001	<0.001	<0.001
Aluminum	0.210	0.016	0.10	Sodium	590	286.8	427.7
Arsenic	0.002	<0.001	<0.001	Sulfate	1000	130	487
Barium	0.146	0.025	0.076	Zinc	0.082	0.003	0.024
Boron	1.40	0.18	0.93	Gross Alpha	-	-	-
Carbonate	12.0	<0.01	1.029	Gross Beta	-	-	-
Bicarbonate	875.4	444	709	Oil & Grease	115.4	12.0	46.6
Cadmium	<0.001	<0.001	<0.001	Phenol	0.008	<0.001	0.005
Calcium	100	5.6	39.7	Alkalinity	-	-	-
Chloride	162	34	131	Acidity	-	-	-
Chromium	<0.001	<0.001	<0.001	Silver	0.002	<0.001	0.001
Copper	0.009	0.004	0.007				
Fluoride	2.90	1.63	1.87				
Iron-Total	3.28	0.064	1.87				
Iron	-	-	-				
Dissolved							
Lead	0.018	<0.001	0.048				
Magnesium	136.8	6.24	56.99				
Manganese	0.46	0.041	0.154				
Mercury	<0.0002	<0.0002	<0.0002				
Molybdenum	-	-	-				
Nickel	-	-	-				

STATION B-6 Price River Above Washing Facility

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	612	4	105	Ammonia	0.40	<0.01	0.14
pH	9.5	7.1	8.2	Nitrate	0.88	<0.01	0.18
Conductivity	1080	315	489	Nitrite	-	-	-
Temp. C	20	0.5	8	Potassium	12.61	0.99	2.05
degrees (water)							
TSS	1556	0.5	164.1	Total	0.320	0.030	0.106
				Phosphate			
TDS	701	194	331	Ortho-	1.07	<0.010	0.089
				Phosphate			
Hardness	278	156	223	Selenium	0.002	<0.001	0.001
Aluminum	0.41	0.009	0.10	Sodium	138	7.00	32.54
Arsenic	0.030	<0.001	0.0015	Sulfate	310	4.0	67.96
Barium	0.22	0.011	0.092	Zinc	0.257	<0.001	0.025
Boron	0.15	0.010	0.067	Gross Alpha	<1	-	-
Carbonate	14.4	<0.01	1.20	Gross Beta	<2	-	-
Bicarbonate	407.48	178.1	255	Oil & Grease	37.0	<1.0	4.09
Cadmium	0.001	<0.001	<0.001	Phenol	0.380	<0.001	0.030
Calcium	68.00	12.0	52.63	Alkalinity	334	146	215
Chloride	26.0	4.0	12.1	Acidity	34	<0.1	10.9
Chromium	0.002	<0.001	0.001	Silver	0.003	<0.001	0.0002
Copper	0.030	<0.001	0.006				
Fluoride	0.30	0.12	0.19				
Iron-Total	25.1	0.015	1.46				
Iron	0.030	0.030	0.030				
Dissolved							
Lead	0.009	<0.001	0.001				
Magnesium	49.4	4.80	24.94				
Manganese	0.872	0.003	0.074				
Mercury	<0.002	<0.002	<0.002				
Molybdenum	-	-	-				
Nickel	-	-	-				

STATION B-5 Price River Below Washing Facility

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	612	1.7	97	Ammonia	0.40	<0.01	0.19
pH	9.6	7.3	8.16	Nitrate	0.57	<0.01	0.19
Conductivity	672	295	475	Nitrite	-	-	-
Temp. C	21	0.2	8.3	Potassium	4.42	1.01	1.85
degrees (water)							
TSS	1628	<1.0	175.6	Total	0.280	0.040	0.123
				Phosphate			
TDS	448	200	317	Ortho-	1.26	<0.010	0.096
				Phosphate			
Hardness	294	162	211	Selenium	0.002	<0.001	0.0001
Aluminum	0.380	0.016	0.09	Sodium	51.40	10.00	25.67
Arsenic	0.025	<0.001	0.001	Sulfate	144	5.0	51.96
Barium	0.31	0.018	0.109	Zinc	0.262	0.002	0.027
Boron	0.175	0.030	0.111	Gross Alpha	<1	-	-
Carbonate	12.0	<0.01	1.029	Gross Beta	<2	-	-
Bicarbonate	361.12	200.08	266.03	Oil & Grease	40.0	<1.0	4.2
Cadmium	0.001	<0.001	<0.001	Phenol	0.307	<0.001	0.029
Calcium	68.8	19.2	53.46	Alkalinity	272	152	228
Chloride	32.0	5.45	13.23	Acidity	30.0	<1.0	8.9
Chromium	0.004	<0.001	0.0003	Silver	0.005	<0.001	0.0002
Copper	0.020	<0.001	0.006				
Fluoride	0.29	0.12	0.19				
Iron-Total	29.0	0.024	1.68				
Iron	0.680	0.040	0.360				
Dissolved							
Lead	0.013	<0.001	0.001				
Magnesium	40.8	3.36	24.63				
Manganese	0.868	0.002	0.079				
Mercury	<0.0002	<0.0002	<0.0002				
			002				
Molybdenum	-	-	-				
Nickel	-	-	-				

STATION B-3 Below Willow Creek Below Mine #6

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	85	0	17.3	Ammonia	-	-	-
pH	9.6	7.4	8.1	Nitrate	0.30	<0.01	0.07
Conductivity	1500	315	813	Nitrite	<0.01	<0.01	<0.01
Temp. C	21	0	8	Potassium	6.420	1.45	2.90
degrees (water)							
TSS	9550	<1.0	1103	Total Phosphate	1.200	.040	0.252
TDS	988	345	575	Ortho- Phosphate	1.70	0.021	0.219
Hardness	-	-	-	Selenium	0.020	<0.001	0.0015
Aluminum	-	-	-	Sodium	139	35.00	69.71
Arsenic	0.020	<0.001	0.003	Sulfate	510	70.5	194.7
Barium	0.880	0.045	0.236	Zinc	0.175	<0.001	0.025
Boron	0.265	0.090	0.144	Gross Alpha	<1	-	-
Carbonate	<0.01	<0.01	<0.01	Gross Beta	<2	-	-
Bicarbonate	409.92	202.52	-	Oil & Grease	10.60	<0.01	1.48
Cadmium	0.002	<0.001	<0.001	Phenol	0.970	<0.001	0.078
Calcium	111.20	33.60	57.33	Alkalinity	352	166	254
Chloride	66.0	11.7	23.87	Acidity	60.0	<0.1	11.4
Chromium	0.040	<0.001	0.003	Silver	0.002	<0.001	0.0003
Copper	0.110	<0.001	0.011				
Fluoride	.45	0.15	0.26				
Iron-Total	431	0.019	36.49				
Iron	0.881	0.040	0.537				
Dissolved							
Lead	0.013	<0.001	0.001				
Magnesium	59.52	7.20	41.51				
Manganese	1.220	0.004	0.140				
Mercury	<0.0002	<0.0002	<0.0 002				
Molybdenum	-	-	-				
Nickel	-	-	-				

STATION B-2 Willow Creek Above Mine #6

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	57	0.04	15	Ammonia	0.30	<0.01	0.16
pH	9.4	7.3	8.2	Nitrate	0.52	<0.01	0.07
Conductivity	1397	328	903.44	Nitrite	-	-	-
Temp. C	24	0	9	Potassium	8.70	1.44	3.64
degrees (water)							
TSS	8770	1.0	553	Total	10.000	0.020	0.567
				Phosphate			
TDS	910	348	631	Ortho-	6.40	<0.010	0.352
				Phosphate			
Hardness	560	248	391	Selenium	0.002	<0.001	0.001
Aluminum	0.001	0.009	0.04	Sodium	126	37.60	79.35
Arsenic	0.016	<0.001	0.003	Sulfate	485	88.0	226
Barium	0.360	0.005	0.108	Zinc	0.145	<0.001	0.021
Boron	0.280	0.050	0.144	Gross Alpha	-	-	-
Carbonate	7.2	<0.01	1.108	Gross Beta	-	-	-
Bicarbonate	434.3	246.44	330.30	Oil & Grease	33.0	<0.01	2.63
Cadmium	0.002	<0.001	<0.001	Phenol	0.340	<0.001	0.002
Calcium	116	19.2	61	Alkalinity	356	202	274
Chloride	39.60	11.4	24.58	Acidity	26.0	<0.01	9.01
Chromium	0.027	<0.001	0.001	Silver	0.004	<0.001	0.0002
Copper	0.090	<0.001	0.013				
Fluoride	.71	0.15	0.32				
Iron-Total	332	0.013	19.36				
Iron	0.58	0.02	0.21				
Dissolved							
Lead	0.008	<0.001	0.001				
Magnesium	78.72	7.68	49.25				
Manganese	1.010	0.003	0.116				
Mercury	<0.0002	<0.0002	<0.0002				
Molybdenum	-	-	-				
Nickel	-	-	-				

STATION Station B-1 Upper Willow Creek

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	76.5	1.8	16.8	Ammonia	-	-	-
pH	9.4	7.1	8.1	Nitrate	0.22	<0.01	0.05
Conductivity	1190	365	784	Nitrite	<0.01	<0.01	<0.01
Temp. C	24	0	8	Potassium	7.20	1.49	2.64
degrees (water)							
TSS	7750	<1.0	701	Total Phosphate	1.720	0.030	0.35
TDS	775	365	570	Ortho- Phosphate	6.10	<0.01	0.069
Hardness	-	-	-	Selenium	0.020	<0.001	0.002
Aluminum	-	-	-	Sodium	142	39.30	77.56
Arsenic	0.125	<0.001	0.01	Sulfate	514	75.0	198.3
Barium	0.30	0.025	0.13	Zinc	0.135	<0.001	0.02
Boron	0.130	0.090	0.11	Gross Alpha	-	-	-
Carbonate	-	-	-	Gross Beta	-	-	-
Bicarbonate	412.36	448.88	332. 71	Oil & Grease	8.4	<0.001	0.02
Cadmium	0.002	<0.001	<0.001	Phenol	0.240	<0.001	0.02
Calcium	124	40.80	61.98	Alkalinity	358	204	277.38
Chloride	70.0	4.0	23.2	Acidity	42.0	<0.01	11.08
Chromium	0.022	<0.001	0.002	Silver	0.002	<0.001	0.001
Copper	0.230	<0.001	0.02				
Fluoride	1.55	0.16	0.35				
Iron-Total	246.0	0.019	20.90				
Iron	0.460	0.010	0.16				
Dissolved							
Lead	0.008	<0.001	0.001				
Magnesium	55.68	7.20	39.37				
Manganese	1.150	0.002	0.17				
Mercury	<0.0002	<0.0002	<0.0 002				
Molybdenum	-	-	-				
Nickel	-	-	-				

STATION BM-28

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	-	-	0.005	Ammonia	0.35	0.05	0.1540
pH	7.9	7.2	7.6080	Nitrate	0.06	0.03	0.2240
Conductivity	1850	1380	1538.	Nitrite	-	-	<0.01
Temp. C	7	4	5.7500	Potassium	7.03	4.76	6.1100
degrees (water)				Total	0.740	0.04	0.1798
TSS	31.0	7.0	14.6	Phosphate	0.043	<0.01	0.0184
TDS	1240	898	1007.	Ortho-	0.004	<0.001	0.0016
Hardness	830	660	776.	Phosphate	0.004	<0.001	0.0016
Aluminum	-	-	-	Selenium	56.0	15.0	28.000
Arsenic	0.014	<0.001	0.0036	Sodium	460	360	404.00
Barium	0.057	0.025	0.0404	Sulfate	0.074	0.012	0.0368
Boron	0.20	0.075	0.1400	Zinc	-	-	-
Carbonate	<0.01	<0.01	<0.01	Gross Alpha	-	-	-
Bicarbonate	488	456.28	467.	Gross Beta	-	-	-
Cadmium	<0.001	<0.001	<0.001	Oil & Grease	7.4	<1.0	3.1800
Calcium	316	180	207.	Phenol	-	-	-
Chloride	32.0	20.0	26.800	Alkalinity	400	374	383.200
Chromium	<0.001	<0.001	<0.001	Acidity	-	-	-
Copper			0.004	Silver	<0.001	<0.001	<0.001
Fluoride	0.44	0.33	0.3840				
Iron-Total	4.390	0.438	2.9244				
Iron							
Dissolved							
Lead	0.004	<0.001	0.0018				
Magnesium	96.48	<0.01	62.0				
Manganese	0.395	0.039	0.2798				
Mercury	<0.0002	<0.0002	<0.0				
Molybdenum	-	-	-				
Nickel	-	-	-				

STATION BM-27

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	0.002	0	0.001	Ammonia	0.36	<0.01	0.2300
pH	8.0	7.63	7.800	Nitrate	0.70	0.06	0.300
Conductivity	1790	1200	1509.	Nitrite	<0.01	<0.01	<0.01
Temp. C	9	9	9.0	Potassium	9.11	5.82	7.7925
degrees (water)				Total	2.30	0.060	0.6638
TSS	14.0	4.0	10.2	Phosphate			
TDS	1200	964	1082.	Ortho-	0.040	0.015	0.0238
Hardness	810	740	762.	Phosphate			
Aluminum			5000	Selenium	<0.001	<0.001	<0.001
Arsenic	<0.001	<0.001	<0.001	Sodium	102	23.0	51.475
Barium	0.070	0.050	0.0550	Sulfate	660	560	590.00
Boron	0.250	0.13	0.1900	Zinc	0.082	0.020	0.0563
Carbonate	<0.01	<0.01	<0.01	Gross Alpha	-	-	-
Bicarbonate	436.7	200	267.	Gross Beta	-	-	-
Cadmium	<0.001	<0.001	<0.001	Oil & Grease	5.2	<1.0	2.7500
Calcium	170.4	152	162.	Phenol			
Chloride	34.0	28.0	31.00	Alkalinity	358	164	219.00
Chromium	<0.001	<0.001	<0.001	Acidity			
Copper				Silver	<0.001	<0.001	<0.001
Fluoride	0.50	0.38	0.4350				
Iron-Total	0.409	0.149	0.3198				
Iron							
Dissolved							
Lead	0.005	<0.001	0.0030				
Magnesium	92.16	84.0	87.2				
Manganese	0.156	0.004	0.0503				
Mercury	<0.0002	<0.0002	<0.0				
Molybdenum			400				
Nickel			002				

STATION BM-26

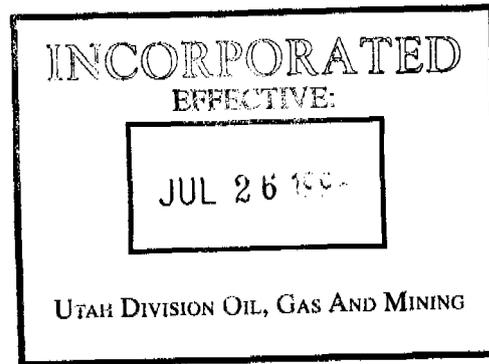
Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	0.01	0.01	0.01	Ammonia	8.00	0.10	7.38
pH	8.2	7.33	7.78	Nitrate	0.84	0.05	0.14
Conductivity	1990	1050	1312	Nitrite	<0.01	<0.01	<0.01
Temp. C	9	8	8.25	Potassium	15.51	6.00	6.65
degrees (water)				Total	2.30	0.150	0.91
TSS	1228	100	633.67	Phosphate			
TDS	1300	700	861.00	Ortho- Phosphate	0.150	0.04	0.09
Hardness	522	470	498.67	Selenium	0.005	<0.001	0.003
Aluminum	<0.001	<0.001	<0.001	Sodium	65	18.5	42.64
Arsenic	<0.001	<0.001	<0.001	Sulfate	640	360	430
Barium	0.055	0.012	0.03	Zinc	0.408	0.043	135.72
Boron	0.30	<0.001	0.18	Gross Alpha	-	-	-
Carbonate	<0.001	<0.001	<0.001	Gross Beta	-	-	-
Bicarbonate	270.8	204.96	228.12	Oil & Grease	13.6	1.7	6.90
Cadmium	<0.001	<0.001	<0.001	Phenol	<0.001	<0.001	<0.001
Calcium	112	88	100.67	Alkalinity	277	168	187
Chloride	32.0	24.0	24.33	Acidity			
Chromium	<0.001	<0.001	<0.001	Silver	<0.001	<0.001	<0.001
Copper	0.005	<0.001	0.003				
Fluoride	0.51	0.36	0.43				
Iron-Total	16.430	0.051	6.00				
Iron Dissolved							
Lead	0.003	<0.001	0.001				
Magnesium	62.4	55.2	59.28				
Manganese	0.227	0.021	0.13				
Mercury	<0.0002	<0.0002	<0.0002				
Molybdenum	0.002	0.002	0.002				
Nickel	<0.001	<0.001	<0.001				

STATION BM-25

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	-	-	-	Ammonia	8.0	0.15	1.4400
pH	8.16	6.9	7.4786	Nitrate	9.00	0.02	1.4786
Conductivity	2050	1390	1646	Nitrite	1.05	1.02	1.0350
Temp. C	13	12	12.50	Potassium	17.80	6.52	10.334
degrees (water)							
TSS	1900	32.0	354. 4286	Total Phosphate	2.80	0.090	0.6451
TDS	1350	906	1080. 5741	Ortho- Phosphate	0.54	0.020	0.2051
Hardness	600	530	565. 4286	Selenium	<0.001	<0.001	<0.001
Aluminum	0.036	<0.001	0.0185	Sodium	185	6.50	137.1429
Arsenic	<0.001	<0.001	<0.001	Sulfate	580	400	362
Barium	0.110	0.005	0.0534	Zinc	0.408	0.011	0.0913
Boron	0.30	<0.001	0.1494	Gross Alpha	-	-	-
Carbonate	19.20	<0.001	2.7514	Gross Beta	-	-	-
Bicarbonate	558.76	268.4	427. 5657	Oil & Grease	4070	3.4	599.2571
Cadmium	<0.001	<0.001	<0.001	Phenol	<0.001	<0.001	<0.001
Calcium	123.2	80.0	7.00	Alkalinity	490	2.54	366.2857
Chloride	150	46.0	99.1 429	Acidity			
Chromium	0.013	<0.001	0.0027	Silver	0.051	<0.001	0.0081
Copper	.005	.004	.0043				
Fluoride	0.66	0.50	0.5743				
Iron-Total	3.42	0.072	1.07 57				
Iron Dissolved							
Lead	.004	<0.001	0.0017				
Magnesium	79.20	60.94	71.5 857				
Manganese	0.218	0.023	0.0734				
Mercury	<0.0002	<0.0002	<0.0 002				
Molybdenum	0.029	<0.001	0.0150				
Nickel	0.169	<0.001	0.0850				

STATION B-43 MC 207

Parameters	Maximum	Minimum	Average	Parameters	Maximum	Minimum	Average
Flow (cfs)	-	-	-	Ammonia	-	-	-
pH	7.783	7.78	7.6125	Nitrate	0.54	0.02	0.3433
Conductivity	1820	1350	167	Nitrite	-	-	-
Temp. C	14	10	11.50	Potassium	11.00	4.50	7.7150
degrees (water)				Total Phosphate	0.800	0.380	.5667
TSS	-	-	363	Ortho-Phosphate	-	-	-
TDS	1679	1120	1024.75	Selenium	<0.001	<0.001	<0.001
Hardness	-	-	-	Sodium	350	310	333.750
Aluminum	-	-	-	Sulfate	619	48.0	258.500
Arsenic	0.240	<0.001	0.0807	Zinc	0.220	0.020	0.0917
Barium	0.100	0.070	0.0820	Gross Alpha	-	-	-
Boron	-	-	0.130	Gross Beta	-	-	-
Carbonate	-	-	-	Oil & Grease	25000	107.20	12553.60
Bicarbonate	1122.40	795.44	979.0500	Phenol	0.048	0.015	0.031
Cadmium	0.003	0.001	0.0017	Alkalinity	920	652	802.50
Calcium	226.4	48.00	104.200	Acidity	148	6.0	49.3500
Chloride	66.00	36.8	54.700	Silver	<0.001	<0.001	<0.001
Chromium	0.007	<0.001	0.0030				
Copper	.080	.014	.0513				
Fluoride	1.53	1.50	1.52				
Iron-Total	4.550	0.120	1.42				
Iron	0.124	0.080	0.10				
Dissolved			20				
Lead	.073	.073	.073				
Magnesium	52.80	16.32	37.6				
Manganese	.140	0.012	0.0930				
Mercury	-	-	<0.002				
Molybdenum	-	-	-				
Nickel	-	-	0.022				



**APPENDIX 7-6**

**SURFACE WATER RAW DATA**



















































































