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**Southern Utah Fuel Company**

a subsidiary of The Coastal Corporation

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041/002 *Pending*  
KEN PAYNE  
Vice President &  
General Manager

October 31, 1990

Darron Haddock  
Permit Supervisor  
Division of Oil, Gas and Mining  
355 West North Temple  
3 Triad Center, Suite 350  
Salt Lake City, Utah 84180-1203

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NOV 05 1990

OIL, GAS & MINING

Dear Mr. Haddock:

Enclosed are 13 copies of the update materials for the 4 East Ventilation Portal Breakout Amendment. The designs have been certified and the riprap design is included in the calculation sheets per your October 18, 1990 Conditional Approval Letter.

I would like to thank you and your staff for the quick review and approval of this amendment.

Sincerely,  
SOUTHERN UTAH FUEL COMPANY

Ken M. Payne,  
Vice President and General Manager

KMP/WKS:jad#128

REVISED 4 EAST VENTILATION PORTALS

INSERTION GUIDE

1. Replace 4 East Ventilation Portal text (2 pages) with new text (3 pages).
2. Replace Maps 1 and 2 with certified Maps 1 and 2.
3. Replace Map 3 dated February 17, 1988 with new Map 3 dated September 1990.
4. Add engineering calculation pages behind text.
5. Add 4 East Portal Drainage Basin Map, Map 4.

WKS:jad#99

#### 4 EAST VENTILATION PORTALS

These ventilation portals will be used as a ventilation intake and an escapeway for quick escape from the 4 East area of the mine in the event the mine requires evacuation and as a site for an exhausting mine fan. The fan is needed for future mining of the northernmost reserves in the Quitchupah lease. Because of this dual purpose design, a small earthen pad will need to be leveled in the breakout area for the fan installation. No water will be discharged from this portal location. Coastal States Energy Company owns the coal and surface on this parcel. Which is located in the SE $\frac{1}{4}$  of the SW $\frac{1}{4}$  of Section 29, T21S, R5E, SLB&M.

A partial print of the Accord Lakes USGS 7 $\frac{1}{2}$  minute quadrangle, Map 1, shows the portal location and surface features. A map of SUFCo's underground workings showing the location of the breakouts is included as Map 2. Map 3 is a detail of the portal entries and breakouts.

Our engineering calculations show that about 700 cubic yards of earth will need to be moved to construct the 0.5 acre fan pad shown on Map 3. Figure 1 shows a cross-section through the pad. The fan will be needed in the fall of 1991. It is necessary to construct the pad during October and November 1990 to allow fan construction during early 1991. Diligent efforts will be expended to minimize the amount of disturbance at the site. The pad will be constructed from within the breakout with underground mining equipment using cut and fill techniques. Because of the thinness of the topsoil, small areal extent of disturbance, and the boulder strewn nature of the site, topsoil will not be collected. Topsoil at the site is categorized as loamy-skeletal, mixed, frigid ustic turriorthent. The pad will be constructed at an elevation of 7,435 feet. Quitchupah creek is at an elevation of about 7,360 feet in this area some 200 feet to the southwest.

Two dry washes traverse through the proposed fan area as shown on Map 3. Runoff from precipitation events and snow melt in these washes will be by-passed through culverts. The 100 year 24-hour event for this area is 2.9 inches. This event was used to size the corrugated metal (CMP) pipe culverts. These culverts will have trash racks to prevent plugging.

The portal location was examined by Dr. Hauck of AERC for possible archeological sites. His report is included as Exhibit I. No sites were found. Endangered Plant Studies, a botanical consulting firm, performed a vegetation and soil survey of the proposed site. Dr. Welsh's report is included as Exhibit II. Exhibit III gives further vegetation and soils data as required by the initial completeness review.

### Sediment Control Practices

A berm will be constructed around the outer perimeter of the pad surface to a minimum height of one foot. The pad will slope to the northeast where runoff will be treated with silt fence and/or straw bales before leaving the site. Out-slopes of the pad will be revegetated during the first fall season after the pad is constructed. During reclamation, hand terracing and silt fence siltation structures will be utilized to control rills and runoff until vegetation is established.

### Right-of-Ways

No special right-of-way surface access is necessary. Construction access will be from within the mine. Electrical power will be routed to the site from East Spring Canyon through the mine such that overhead power lines to the site will not be necessary.

### Sealing upon Final Abandonment

A breakout seal will be constructed in each of the breakouts from the inside as shown on the Typical Portal Seal drawing presented in Volume 3 of the M&RP on page 216. These seals will be of a substantial design and constructed of concrete block utilizing a waterproof sealant such that the seal will withstand the hydraulic head that could develop if the entire mine was inundated. Since this work will be done many years in the future, the design will be reviewed with the regulatory authority before such sealing is undertaken. The best economically feasible technology will be used. A currently acceptable design is described below.

In compliance with 30 CFR 75.1711-2, seals will be installed in the entry as soon as mining is completed and the mine is to be abandoned. Prior to installation, all loose material within three feet of the seal area will be removed from the roof, rib, and floor. The mine entry seal will be made of solid concrete blocks (average minimum compressive strength of 1,800 psi; tested in accordance with A.S.T.M. C-140-70) and mortar (one part cement, three parts sand, and no more than seven gallons of water per sack of cement).

The seal will be installed in the following manner: The seal will be recessed at least 16 inches deep into the rib and 12 inches deep into the floor. No recess will be made into the roof. The blocks will be at least six inches high except on the top course, and eight inches wide. The blocks will be laid and mortared in a transverse pattern. In the bottom course, each block will be laid with the long axis parallel to the

rib. The long axis in succeeding courses will be perpendicular to the long axis block in the preceding course. An interlaced pilaster will be constructed in the center. The seals will have a total thickness of at least 16 inches. The entry will then be backfilled and graded to the approximate slope of the area surrounding the portal entry. For details, see Figures 783.13/A and 783.18/B.

### Reclamation

The 4 East breakout area will consist of two portals located on a south-facing slope in the pinon-juniper community type. This community is very similar to the other portal sites. Vegetation and soils information are contained in Exhibit II. The disturbed area at the site will be so small as to create minimal disturbance to the surrounding vegetative communities.

Reclamation activities will consist of removing both the 24 inch and 72 inch CMP's. The ephemeral stream channels will be returned to trapezoidal cross-sections that will pass the 100 year 6-hour precipitation event. Such a cross section for the main drainage would have a bottom width of 10 feet with side slopes of 2h:1v and a depth of 2 feet. The smaller drainage would have a bottom width of 3 feet with side slopes of 2h:1v and a depth of 1 foot. Riprap with a D<sub>50</sub> of 1 foot will be used for the main drainage. The underlying filter blanket will be 23 inches thick composed of a well graded gravel. The smaller drainage will have a 6 inch thick filter blanket and riprap with D<sub>50</sub> of four inches. After the CMP's have been removed the portal openings will be backfilled and sealed. After sealing and burial of the portal openings, scarification of the slope by hand raking will take place. Then the appropriate amounts of the seed mix given in Exhibits II and III will be planted. Seeding will take place during late fall for optimal success. Establishment of shrub species will take place by natural reinvasion.

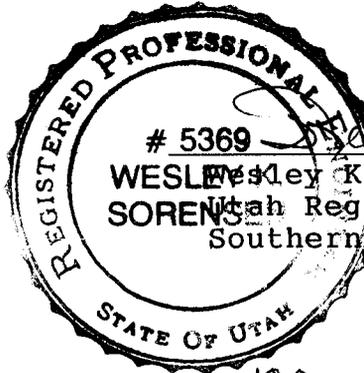
WKS:jad#96

Revised 10/31/90

CERTIFICATION STATEMENT

The hydrologic calculations for the 4 East Portal consisting of pages 1-8 were performed using standard engineering methods by Wesley K. Sorensen, P.E. These calculations represent sound design practice and are correct to the best of my knowledge.

Maps 1-4 accurately represent the layout of the proposed 4 East Portal and drainage structures.



10/31/90

WKS 1/5

9/6/90

Runoff Calculations - 4 EAST PORTAL  $\rightarrow$  10/11/90  
DRY WASH

$P = 2.9$  in for 100 yr - 24 hr

$CN \approx 75$

Area = 445.82 acres

$$S = \frac{1000}{CN} - 10$$

$$S = \frac{1000}{75} - 10$$

$$S = 3.33$$

$$Q = \frac{(P - 0.2S)^2}{P + 0.8S}$$

$$Q = \frac{(2.9 - 0.2(3.33))^2}{2.9 + 0.8(3.33)}$$

$$Q = 0.897 \text{ inches}$$

Time of concentration,  $t_c$

$L = 1519$	$8801 \rightarrow 8560$	$d = 1500$ ft	$s = 16\%$ grass - sparse	$v \approx 4$ fps
$L = 4202$	$8560 \rightarrow 8440$	$d = 4200$ ft	$s = 2.9\%$ grass & sage	$v \approx 1.3$ fps
$L = 2775$	$8440 \rightarrow 7800$	$d = 2700$ ft	$s = 23.7\%$ pinon - juniper	$v \approx 5$ fps

$$t_c = \sum_{i=1}^3 \frac{L_i}{V_i} \quad \text{where } L_i = \sqrt{d_i^2 + h_i^2}$$

$$t_c = 69.45 \text{ minutes}$$

$$t_c = 1.16 \text{ hrs}$$

$$g_p' = 290 \text{ cfs/mi}^2/\text{in} \quad \text{from fig 2.40}$$

A HSFDA pg 115

$$BP = g_p' A Q \quad A = \text{area mi}^2$$

$$Q = \text{runoff volume inches}$$

$$BP = 290(445.8)(0.897)/640$$

$$BP = \underline{\underline{18 \text{ cfs}}}$$

### Culvert Design

ASSUME HEAD WALL & WING WALLS square-edged concrete w/ trash rack @ 60°.

$$\frac{HW}{D} = 1.0$$

From Figure 4-28 inlet control H.B. steel  
 Dia = 72" Length = 210ft S = 10% drainage  
 or 7'-0" x 5'-1" squash

from Fig 4-30 outlet control H.B. S.D.  
 H = 1.9 ft with culvert flowing full.  
 TW = 1.33 ft pg 4/5

MAKE INLET AT LEAST 2 ft higher than culvert.

$$\text{FOR } 72" \quad \text{Area} = 28.27 \text{ ft}^2 \Rightarrow V = 6.4 \text{ ft/sec}$$

$$\text{FOR } 7'-0" \times 5'-1" \quad \text{Area} = 26.00 \text{ ft}^2 \Rightarrow V = 6.96 \text{ ft/sec}$$

$$HW = H + TW - L_{50} - \frac{V^2}{2g}$$

$$HW < 0 \Rightarrow \underline{\underline{\text{inlet control}}}$$

Runoff Calculations - small wash near  
prospect workings

$P = 2.9$  in for 100 yr - 24 hr event

$CN = 75$

Area = 12.85

$Q = 0.897$  in see dry wash calc.

Time of concentration

$L = 1124$  8400 - 7800  $d = 950$  ft  $s = 63\%$  pinyon-rock  $n = 0.8$

$t_c = 141$  seconds

$t_c = 0.39$  hrs

$\beta_p' = 600$  csm/inch Fig 2.40 AHSFDA pg 115

$g_p = (600)(12.85)(0.897)/640$

$g_p = \underline{10.81}$  cfs.

culvert design

Assume head wall & wingwalls square-edged  
concrete w/ trash rack @  $60^\circ$

$\frac{HW}{D} = 1.0$

From Figure 4-28 inlet control H.B. STEEL

Dia = 24" length = 140 ft  $s = 0.10$  drainage

FROM Fig 4-30 outlet control H.B.S.D.

$L' = L \left( \frac{n'}{n} \right) = L' = 140(0.44) = 62$  ft

$H = 0.7$  ft  $\Rightarrow$  MAKE HEAD WALL AT LEAST 1 FT

Exit velocity for 24":  $A = 3.142 \text{ ft}^2$

$$V = \frac{Q}{A}$$

$$V = \frac{10.81}{3.142}$$

$$V = 3.44 \text{ ft/sec}$$

$$HW = H + TW - LS - \frac{V^2}{2g}$$

$$HW = 0.7 + 1.33 - (140)(.10) - \frac{3.44^2}{64.4} = HW < 0 \Rightarrow \underline{\underline{\text{INLET CONTROL}}}$$

Energy Dissipator

Note: Both 72" & 24" will discharge into same dissipator.

$$Q_{72} = 181 \text{ cfs}$$

$$Q_{24} = 11 \text{ cfs}$$

$$Q_T = 192 \text{ cfs}$$

Channel downstream of dissipator has a slope of 16%. Slope transition into natural channel at 5%. Using a trapezoidal channel  $b = 10 \text{ ft}$ ,  $z = 2$ , depth = 2 ft.

From notes 10/11/90.  $Q = 192 \text{ cfs}$   $d = 1.33 \text{ ft}$

TAIL WATER DEPTH  $TW = d = 1.33 \text{ ft}$

Brink depth  $Y_0$

$$\frac{Q}{D^{2.5}} = \frac{191}{6^{2.5}} = 2.17$$

$$\frac{TW}{D} = \frac{1.33}{6} = 0.22$$

From Eq 7.21  $V_n = 0.9 \text{ ft}$

Use a CSU disapator design per Redbook pg 535 Fig 7.22

USE  $d_{50} = 1 \text{ ft}$

$h_s = 1 \text{ ft}$

$Y_0 = 0.9 \text{ ft}$

Length = 18 ft of disapator pool

Apron length = 6 ft

FINAL CHANNEL DESIGN

P = 1.8 in for 100yr 6hr event.

CN = 75

Area = 445.82 acres (MAIN)

A = 12.85 TRIB

$S = \frac{1000 - 10}{75}$

$S = 3.33$

$Q = \frac{(P - 0.2S)^2}{P + 0.8S}$

$Q = \frac{[1.8 - 0.2(3.33)]^2}{1.8 + 0.8(3.33)}$

$Q = 0.288 \text{ inches}$

FOR MAIN  $t_c = 1.16 \text{ hrs}$  see pg 1 of calcs.

FOR TRIB  $t_c = 0.39 \text{ hrs}$  see pg 3 of calcs

$q_{\text{main}} = 290(445.8)(0.288)/640 = \underline{58.2 \text{ cfs}}$

From pg 7  $v \hat{=} 10 \text{ fps}$  From rip rap curve  $D_{50} = 15 \text{ inch}$

$q_{\text{trib}} = 600(12.85)(0.288)/640 = \underline{3.5 \text{ cfs}}$

From pg 7  $v \hat{=} 5 \text{ fps}$  From rip rap curve  $D_{50} = 4 \text{ inch}$

Filter blanket of sand or road base Main = 23 inch

OPEN CHANNEL FLOW  
4 EAST FAN PORTAL

WKS  
10/11/90

	100 YR
QUANTITY	192.00
S	0.050
AREA	16.84
V	11.40
DEPTH	1.330
n	0.030

FOR TRAPEZODIAL		DESIGN
B (BOTTOM WIDTH)	10.00	10
Z (SIDE SLOPE)	2.00	2
A (AREA)	16.84	28.00
R	1.00	1.48
DEPTH	1.33	2

RGT HAND SIDE	193.89
QUANTITY	

W.K. SORENSEN 31-Oct-90

7

OPEN CHANNEL FLOW  
4 EAST PORTAL DRANIAGES UPON ABANDONMENT  
100 YR 6 HR

	MAIN TRIBUTARY	
QUANTITY	58.20	3.50
S	0.100	0.100
AREA	6.01	0.70
V	9.69	4.98
DEPTH	0.542	0.206
n	0.030	0.030

FOR TRAPEZODIAL		
B (BOTTOM WIDTH)	10.00	3.00
Z (SIDE SLOPE)	2.00	2.00
A (AREA)	6.01	0.70
R	0.48	0.18

RGT HAND SIDE	58.13	3.51
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LOW				
) = pipe diameter.				
.06	.07	.08	.09	
.02	.02	.03	.04	
.08	.09	.10	.10	
.16	.17	.18	.19	
.25	.26	.27	.28	
.35	.36	.37	.38	
.45	.46	.47	.48	
.55	.56	.57	.58	
.64	.65	.66	.67	
.72	.72	.73	.74	
.78	.78	.78	.78	

rip energy dissipators is based on University (FHA, 1975b). It is that scour hole geometry (Q), brink depth (Y<sub>o</sub>), and it is noted that the riprapped scour energy dissipator if the ratio is less than 0.75. For greater ratios through the basin creating a channel through the downstream channel re-

in Figure 7.22. The basin surface of the riprapped floor is to be a vertical exit. The ratio of scour is to be between 2 and 4. Energy dissipator whichever is greater and the ratio of the channel width to the riprapped floor width for a box

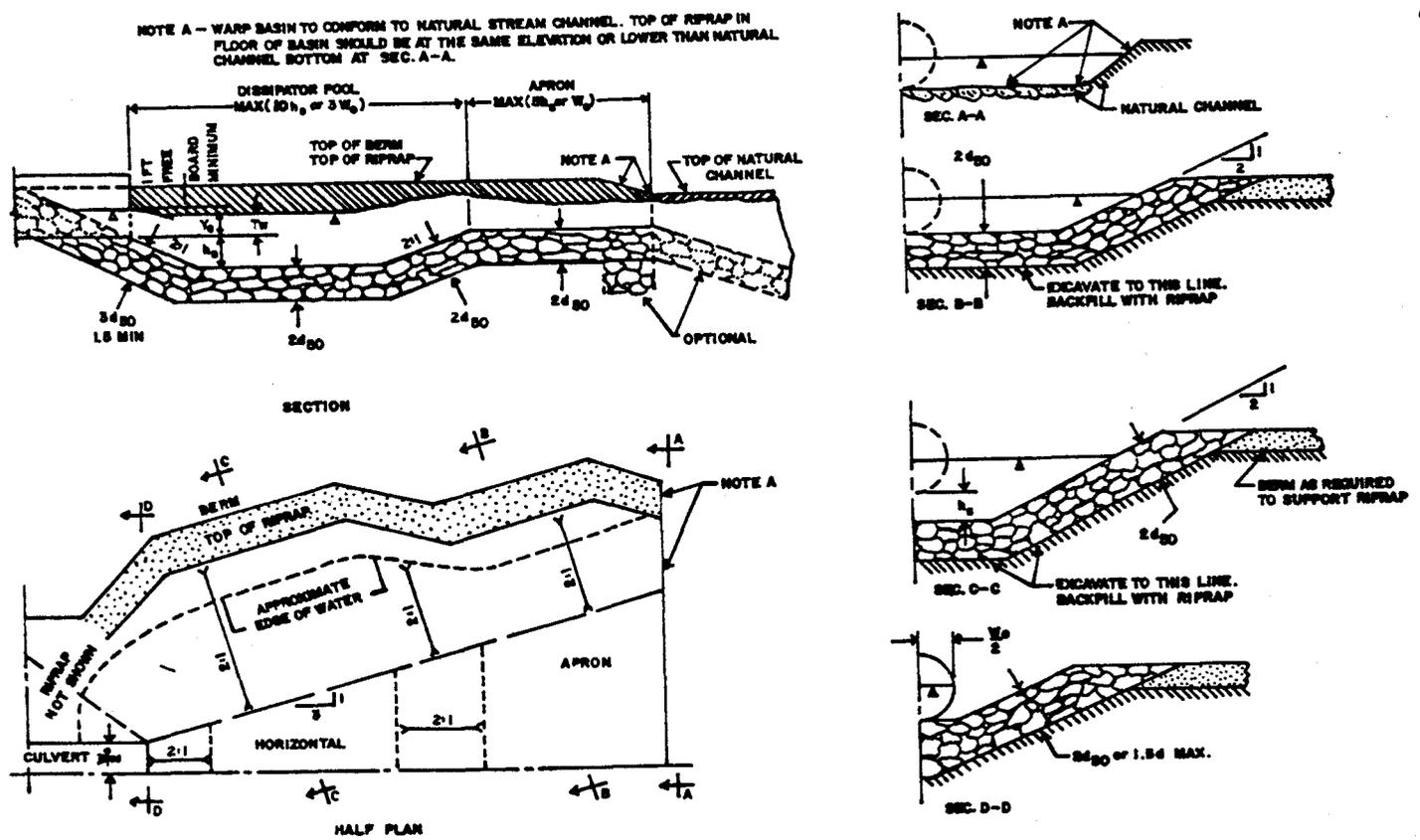
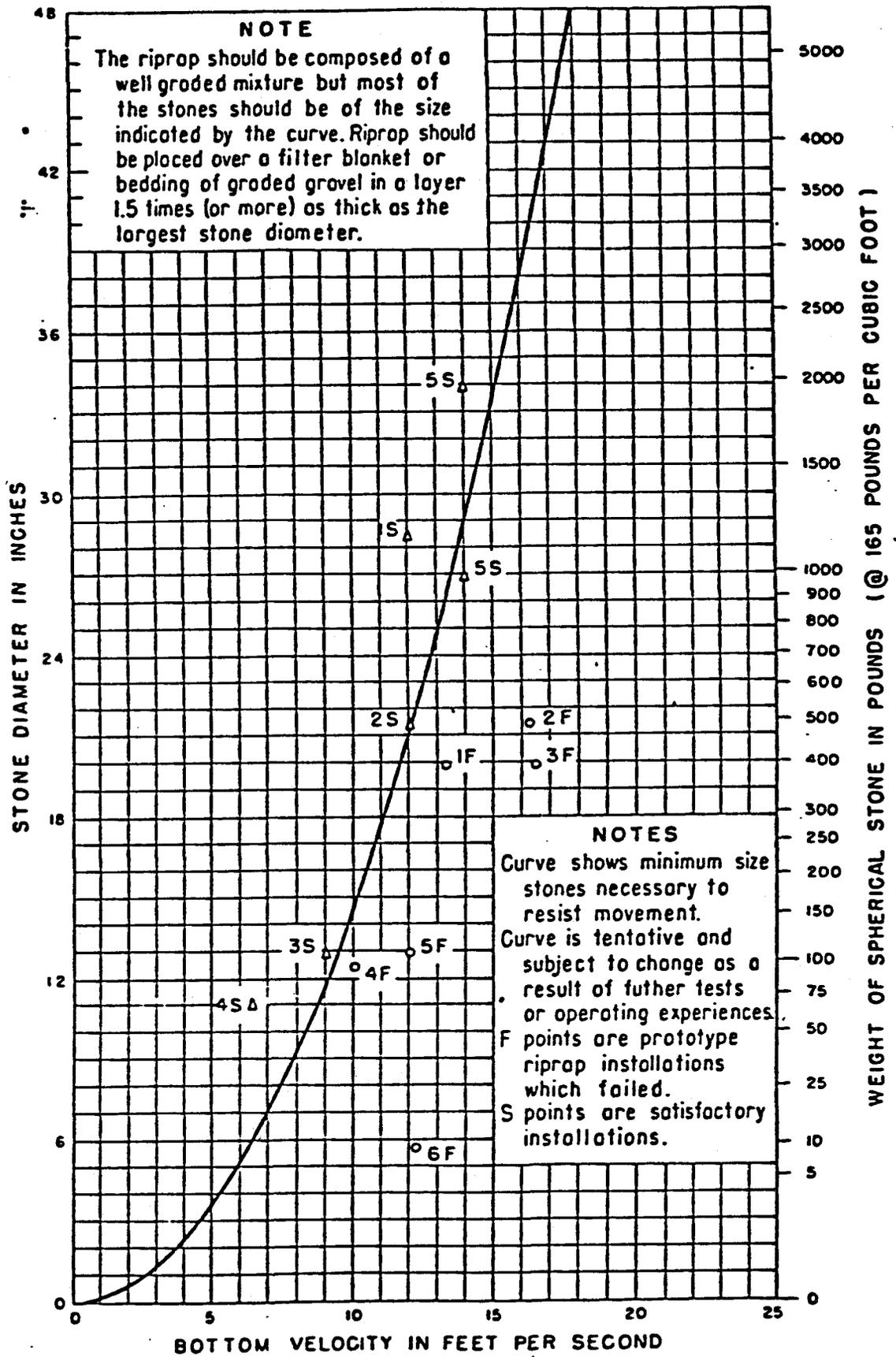
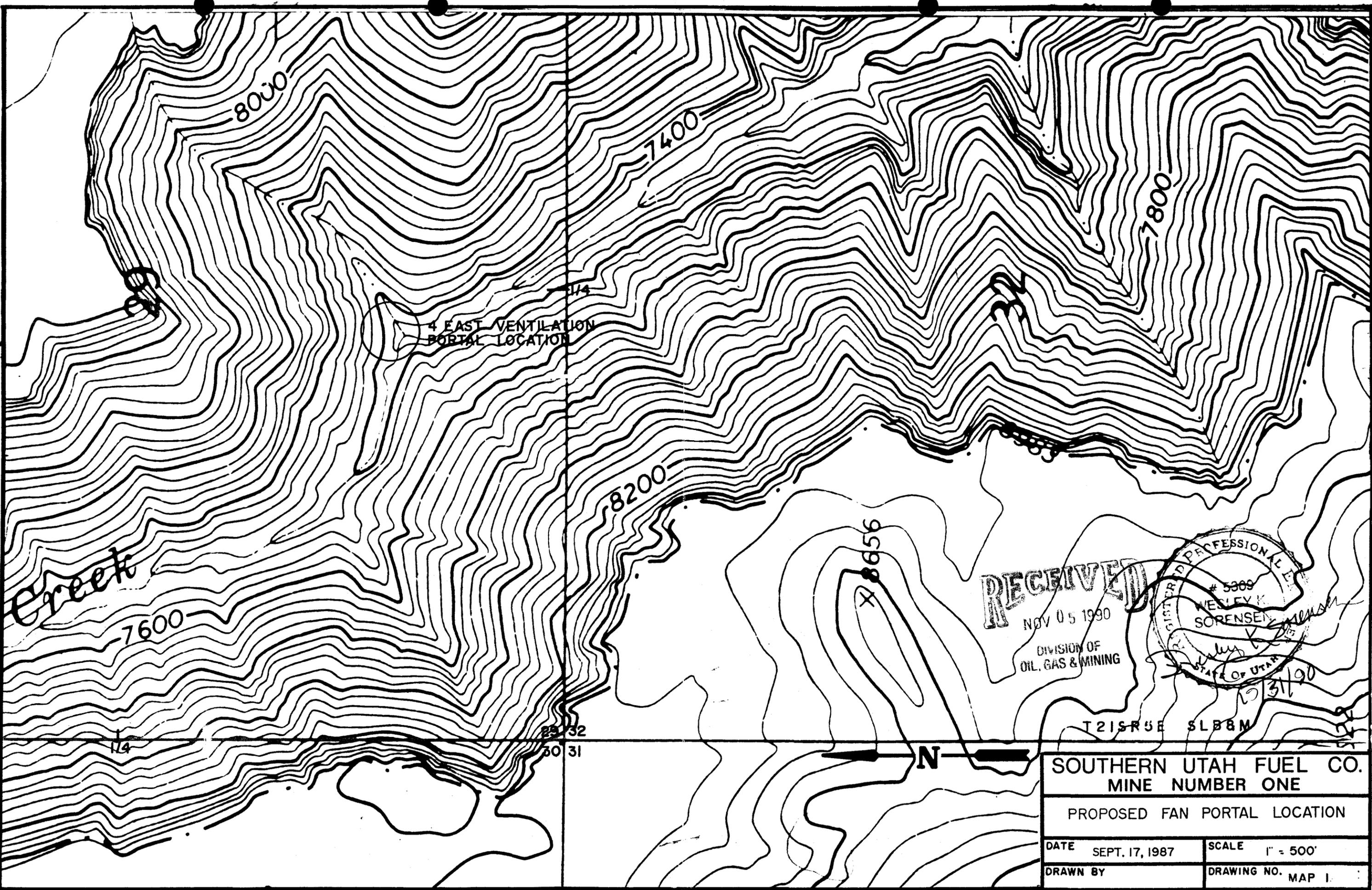


Figure 7.22. Schematic of rip-rapped culvert energy basin. (adapted from FHA, 1975b)



Curve to determine maximum stone size in riprap mixture



4 EAST VENTILATION PORTAL LOCATION

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REGISTERED PROFESSIONAL ENGINEER  
# 53009  
WESLEY K. SORENSEN  
STATE OF UTAH  
10/31/90

T21SR5E SLB8M

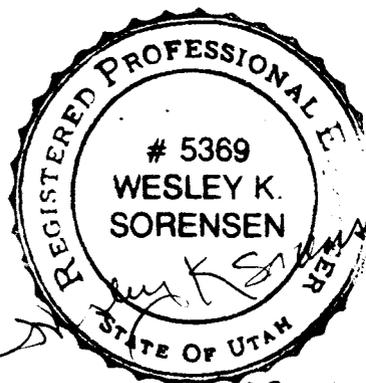


SOUTHERN UTAH FUEL CO.  
MINE NUMBER ONE

PROPOSED FAN PORTAL LOCATION

DATE	SEPT. 17, 1987	SCALE	1" = 500'
DRAWN BY		DRAWING NO.	MAP 1





10/31/90

DRY WASH

FAN PORTAL

INTAKE PORTAL

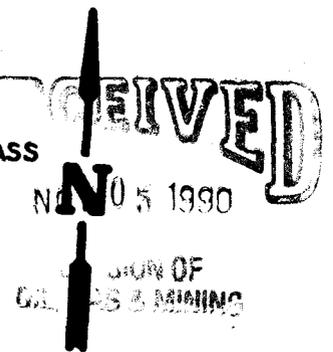
CUT BOUNDARY

72 in. BYPASS  
C.M.P.

TOE OF  
FILL

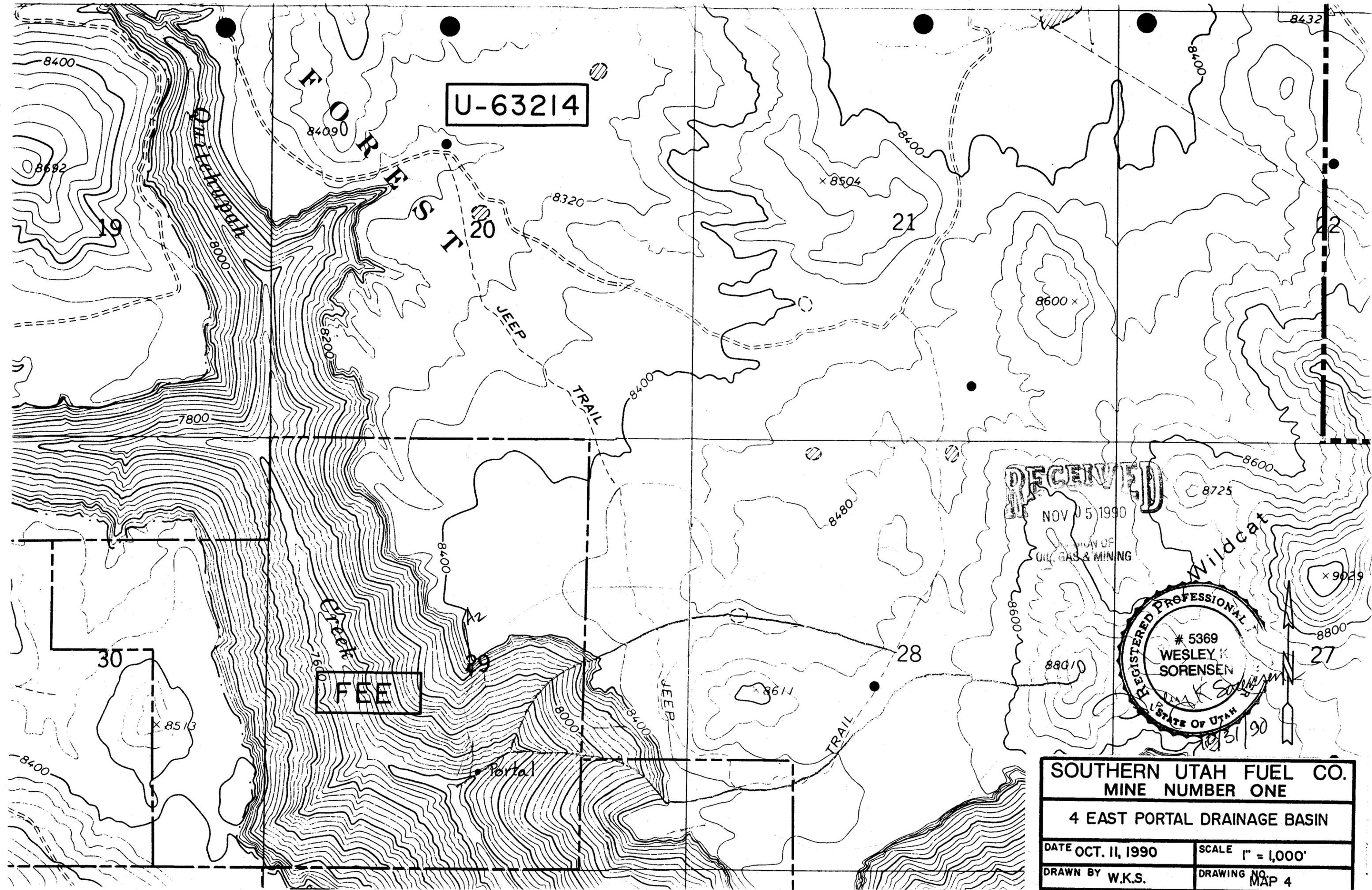
PAD BERM

24 in. BYPASS  
C.M.P.



**SOUTHERN UTAH FUEL CO.  
MINE NUMBER ONE  
4 EAST VENTILATION  
PORTAL**

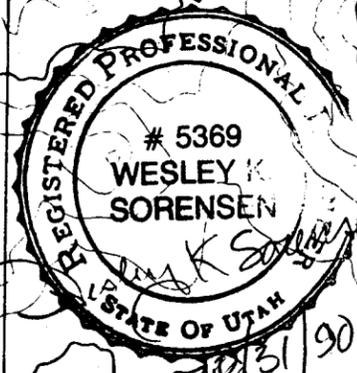
DATE	SEPT. 1990	SCALE	1" = 100'
DRAWN BY	S.K.S.	DRAWING NO.	MAP 3



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SOUTHERN UTAH FUEL CO.  
MINE NUMBER ONE

4 EAST PORTAL DRAINAGE BASIN

DATE OCT. 11, 1990

SCALE 1" = 1,000'

DRAWN BY W.K.S.

DRAWING NO. MAP 4